

070001 Washington Bridge North

Inspected By Inspector:

Last Inspection Date

07/22/2020

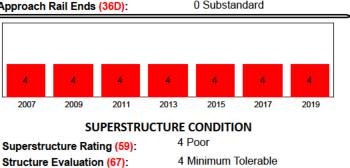
AECOM

(IDENTIFICATION	INSPECTION
Bridge ID:	070001	Date of Inspection (90): 7/22/2020
NBI Number:	Washington Bridge North	Frequency (91): 24
Structure Name:	Washington Bridge North	Next Inspection: 7/24/2021
Location (9):	0.2 Mi W of JCT US 6	Inspection Type Freq (92) Last Insp (93) Next Insp
Carries (7):	I-195 WB	Element 12 7/22/2020 7/24/2021
Type of Service (42A):	1 Highway	Fracture Critical (A) 1/1/1901 1/1/1901
Feature Crossed (6):	SEEKONK RIVER	Underwater (B) 48 7/24/2017 7/24/2021
Type of Service (42B):	8 Hwy-waterway-RR	Special Insp (C) 12 7/22/2020 7/24/2021
Placecode (4):	East Providence	
County (3):	Providence	LOAD RATING AND POSTING
State (1):	44 Rhode Island	Posting Status (41) A Open, no restriction
Station:	NBI	Posting % (70): 5 At/Above Legal Loads Rating Date: 1/19/2018
Region (2):	District 3	Rating Date.
Latitude :	41.8200000	Design Load (31): 6 MS18(HS20)+mod
Longitude :	-71.3900000	Opr Method (63): 8 LRFR (HL93)
Owner (22):	01 State Highway Agency	Opr Rating (64): 52.00 Tons
Custodian (21):	01 State Highway Agency	Inv Method (65): 8 LRFR (HL93)
Year Built (27):	1969 Border State: Not Applicable (P)	
Year Recon (106):	1998 Border Number:	
	ot eligible for NRHP % Responsibility:	

	EOMETRY							
Deck Geometry (68):	4 Tolerable							
Deck Area:	145,531.82	6	6	6	6	6	6	6
Deck Type (107):	1 Concrete-Cast-in-Place							
Wearing Surface (108A):	6 Bituminous	2007	2009	2011	2013	2015	2017	2019
Membrane (108B):	2 Preformed Fabric			DEC	K CONDI	TION		
Deck Protection (108C):	8 Unknown	Deck Rati	ng (58):			6 Satisfa	ctory	
O. to O. Width (52):	76.44	Bridge Ra	il (36A):			1 Meets S	Standards	
Curb / Sidewalk Width L (50A):	0.00	Transition	(36B):			0 Substa	ndard	
Curb / Sidewalk Width R (50B):	0.00	Approach	Rail (36C)	:		0 Substa	ndard	
Median (33):	0 No median	Approach	Rail Ends	(36D):		0 Substa	ndard	

SUPERSTRUCTURE	GEOMETRY
JOFENJIKOCIONE	GLOWILTKI

# of Main Spans (45):	1
# of Approach Spans (46):	20
Main Material (43 A):	3 Steel
Main Design <mark>(43 B)</mark> :	02 Stringer/Girder
Max Span Length (48):	130.60
Structure Length (49):	1,903.87
NBIS Length (37):	Long Enough
Temp Structure (103):	Not Applicable (P)
Skew (34):	0
Structure Flared (35):	1 Yes, flared
Parallel Structure (101):	Left of bridge
Approach Alignment (72):	6 Equal Min Criteria



Driven to get you there		RIDOT Bridg spection Rep	port In	070001 Washington Bridge North aspected By AECOM Inspector: ast Inspection Date 07/22/2020
SUI Navigation Control (3 Nav Vert Clearance (Pier Protection (111): Lift Bridge Vertical Clearance (116): Scour Rating (113): Waterway Adequacy (9): 137.75 (40): 327.23 2 In-Pl 3 SC -	Not Required	2007 2009 2	5 4 4 4 4 011 2013 2015 2017 2019 STRUCTURE CONDITION): 6 Satisfactory 6 Bank Slumping
1ST ROUTE UNDER ROADWAY Pos Prefix (5A): Kind of Hwy (5B): Route Num (5D): LRS Route (13A/B): Milepost (11): Suffix (5E): Lanes Under (28B): Detour Length (19):		ROADWAY Funct Class (26): Level Service (5C): NHS (104): Defense Hwy (100): Toll Facility (20): ADT (29): Pct Trucks (109): ADT Year (30):	CLASSIFICATION 17 Urban Collector 1 Mainline 0 Not on NHS 0 Not a STRAHNET hwy 3 On free road 0 Cars/Day 0.00% 2000	CLEARANCESVertical (10):14.83Min Vert Over (53):18.3314.17Vert Ref (54A):H Hwy beneath structHorizontal (47):82.50Min Lat Left (56):0.00Min Lat Right (55B):19.69Horiz Ref (55A):H Hwy beneath structUnderclearance (69):4 Tolerable
2ND ROUTE UNDE ROADWAY I Pos Prefix (5A): Kind of Hwy (5B): Route Num (5D): LRS Route (13A/B): Milepost (11): Suffix (5E): Lanes Under (28B): Detour Length (19):		ROADWAY Funct Class (26): Level Service (5C): NHS (104): Defense Hwy (100): Toll Facility (20): ADT (29): Pct Trucks (109): ADT Year (30):	CLASSIFICATION 19 Urban Local 2 Alternate 0 Not on NHS 0 Not a STRAHNET hwy 3 On free road 0 Cars/Day 0.00% 2000	CLEARANCESVertical (10):25.00Min Vert Over (53):18.3314.17Vert Ref (54A):H Hwy beneath structHorizontal (47):40.60Min Lat Left (56):0.00Min Lat Right (55B):19.69Horiz Ref (55A):H Hwy beneath structUnderclearance (69):4 Tolerable
3RD ROUTE UNDE ROADWAY Pos Prefix (5A): Kind of Hwy (5B): Route Num (5D): LRS Route (13A/B): Milepost (11): Suffix (5E): Lanes Under (28B): Detour Length (19):		ROADWAY Funct Class (26): Level Service (5C): NHS (104): Defense Hwy (100): Toll Facility (20): ADT (29): Pct Trucks (109): ADT Year (30):	CLASSIFICATION 19 Urban Local 2 Alternate 0 Not on NHS 0 Not a STRAHNET hwy 3 On free road 0 Cars/Day 0.00% 2000	CLEARANCESVertical (10):21.00Min Vert Over (53):18.3314.17Vert Ref (54A):H Hwy beneath structHorizontal (47):43.30Min Lat Left (56):0.00Min Lat Right (55B):19.69Horiz Ref (55A):H Hwy beneath structUnderclearance (69):4 Tolerable



Bridge Condition Poor

070001 Washington Bridge North

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ROADWAY	LOCATION	ROADWAY	CLASSIFICATION	CLEA	ARANCES	
Pos Prefix <mark>(5A)</mark> : Kind of Hwy <mark>(5B)</mark> :	4th Route Under 5 City Street	Funct Class (26): Level Service (5C):	19 Urban Local 2 Alternate	Vertical <mark>(10)</mark> : Min Vert Over <mark>(53)</mark> :	14.20 18.33	14.1
Route Num (5D):	0	NHS (104):	0 Not on NHS	Vert Ref (54A):	H Hwy beneath s	truct
LRS Route (13A/B):		Defense Hwy (100):	0 Not a STRAHNET hwy	Horizontal (47):	35.40	
Milepost (11):		Toll Facility (20):	3 On free road	Min Lat Left (56):	0.00	
Suffix (5E):	0 N/A (NBI)	ADT (29):	0 Cars/Day	Min Lat Right (55B):	19.69	
Lanes Under (28B):	2	Pct Trucks (109):	0.00%	Horiz Ref (55A):	H Hwy beneath s	truct
Detour Length (19):	0.00 mi (0.00 km)	ADT Year (30):	2000	Underclearance (69	: 4 Tolerable	

ROADWAY	LOCATION	ROADWAY	CLASSIFICATION	CLEA	RANCES	
Pos Prefix (5A):	Route On Structure	Funct Class (26):	11 Urban Interstate	Vertical (10):	99.99	
Kind of Hwy (5B):	1 Interstate Hwy	Level Service (5C):	1 Mainline	Min Vert Over <mark>(53</mark>):	18.33	14.17
Route Num (5D):	00195	NHS (104):	1 On the NHS	Vert Ref <mark>(54A)</mark> :	H Hwy beneath stru	ict
LRS Route (13A/B):	6700-A/00	Defense Hwy (100):	1 On Interstate STRAHNET	Horizontal (47):	59.71	
Milepost <mark>(11</mark>):	2.42 mi (3.90 km)	Toll Facility (20):	3 On free road	Min Lat Left <mark>(56)</mark> :	0.00	
Suffix (5E):	4 West	ADT <mark>(29</mark>):	76,700 Cars/Day	Min Lat Right (55B):	19.69	
Lanes On (28A):	5	Pct Trucks (109):	10.00%	Horiz Ref <mark>(55A)</mark> :	H Hwy beneath stru	ict
Detour Length (19):	2.00 mi (3.22 km)	ADT Year (30):	2008	Underclearance (69)	: 4 Tolerable	

BRIDGE NOTES

Orientation:

The main bridge structure carries I-195 Westbound and consists of eighteen (18) spans labeled Span #1 through #18. The spans are logged west to east with Girder 'A' at the north fascia.

The Gano Street Ramp ties into the main bridge structure at the north side of Span #5 and consists of three (3) spans labeled Span #1R through #3R. The spans are logged west to east with Box Girder Cell 'A' at the south (true west) fascia.

The Seekonk River flows north to south below the structure.

Equipment:

60' manlift, 60' bucket boat, Ladder and Air Monitor.

Traffic Control:

Lane Closures on Gano Street (Span #1), Water Street (Span #15), Waterfront Drive (Span #16) and Valley Street (S pan #18) with local police details. Moving closure on I-195 Westbound with state police details for topside inspection.

Access Notes:

- Access to the underside of Span #10 through Span #14 requires access to the CARDI construction yard. Check in with local personnel on site.

- The boat was launched from East Providence Yacht Club dock on Pier Road in East Providence .

- The interior of the Gano Street Ramp box girders was accessed through the hatches at West Abutment #1R with a 24' ladder (Photos 76, 77). The key for the box girder hatches can be obtained from David Cluley at the RIDOT Bridge Inspection office on Jefferson Boulevard. The access hatch to Cell 'C' is jammed and remains partially open allowing pigeons access to the box girder interior (Photo 77).

- The catwalks on the interior portions of Pier #6 and Pier #7 can be accessed through hatches and ladders on the topside of the north overhang (Photo 141).

- The electrical utility room in the East Abutment has a locked door. The lock key can be obtained from David Cluley at the RIDOT Bridge Inspection office on Jefferson Boulevard.

INSPECTION NOTES



070001 Washington Bridge North

Inspected By

Inspector: Last Inspection Date AECOM 07/22/2020

Bridge Condition Poor

Routine and Special Inspection by AECOM Inspection Date: Multiple dates from 06/29/20 to 7/22/20

Weather: 80° - 95° Fahrenheit

Special Inspection Requirements:

The special inspection includes the superstructure and substructure.

NBI Ratings:

The bridge is in overall Poor condition. The condition rating for Item 60 - Substructure has been increased from (4 - Poor) to (6 - Satisfactory) based on the repairs which have been made throughout the bridge substructure elements. The condition ratings for the Item 58 – Deck (6 - Satisfactory) and Item 59 – Superstructure (4 - Poor) remain unchanged since the last inspection.

Bridge Construction:

There is scaffolding in place throughout the structure (from previous bridge rehabilitation construction) allowing access to the drop-in girder ends and corbels (Photos 17, 18, 121, 125, 135, 145). There is typical construction debris scattered through the scaffolding (Photos 146, 147). There is typical construction wiring in place throughout the bridge.

For additional inspection notes refer to the attached file "070001 Additional Inspection Notes.pdf".

	ional inspection notes re	r i i	r I	-					F	-
Elm/Env	Description	Total Qty	% in 1	Qty. St. 1	% in 2	Qty. St. 2	% in 3	Qty. St. 3	% in 4	Qty. St. 4
12/3	Re Concrete Deck	142,889.0	94%	134,317.00	5%	7,144.00	1%	1,428.00	0%	0.00
510/3	Wearing Surfaces	142,889.00	94%	134,317.00	<mark>5%</mark>	7,144.00	1%	1,428.00	0%	0.00
3210/3	DeVSpal/Patch/Pol(Wear Surf)	4,286.00	0%	0.00	83%	3,572.00	17%	714.00	0%	0.00
3220/3	Crack (Wearing Surface)	4,286.00	0%	0.00	83%	3,572.00	17%	714.00	0%	0.00
1080/3	Determination/Spail/Patched Area	2,143.00	0%	0.00	83%	1,786.00	17%	357.00	0%	0.00
1090/3	Exposed Rebar	2,143.00	0%	0.00	83%	1,786.00	17%	357.00	0%	0.00
1120/3	Efforescence/Rust Staining	2,143.00	0%	0.00	83%	1,786.00	17%	357.00	0%	0.00
1130/3	Cracking (RC and Other)	2,143.00	0%	0.00	83%	1,786.00	17%	357.00	0%	0.00
16/3	Re Conc Top Flange	7,336.00	81%	5,911.00	16%	1,150.00	4%	275.00	0%	0.00
510/3	Wearing Surfaces	7,336.00	100%	7,336.00	0%	0.00	0%	0.00	0%	0.00
1080/3	Determination/Spail/Patched Area	200.00	0%	0.00	100%	200.00	0%	0.00	0%	0.00
1090/3	Exposed Rebar	25.00	0%	0.00	0%	0.00	100%	25.00	0%	0.00
1120/3	Efflorescence/Rust Staining	1,000.00	0%	0.00	75%	750.00	25%	250.00	0%	0.00
1130/3	Cracking (RC and Other)	200.00	0%	0.00	100%	200.00	0%	0.00	0%	0.00
105/3	Re Clsd Box Girder	922.00	8%	78.00	55%	505.00	37%	339.00	0%	0.00
1080/3	Determination/Spail/Patched Area	100.00	0%	0.00	80%	80.00	20%	20.00	0%	0.00
1090/3	Exposed Rebar	5.00	0%	0.00	0%	0.00	100%	5.00	0%	0.00
1120/3	Efflorescence/Rust Staining	244.00	0%	0.00	50%	122.00	50%	122.00	0%	0.00
1130/3	Cracking (RC and Other)	495.00	0%	0.00	61%	303.00	39%	192.00	0%	0.00
107/3	Steel Opn Girder/Beam	1,320.00	60%	787.00	38%	496.00	3%	37.00	0%	0.00
515/3	Steel Protective Coating	19,385.00	38%	7,350.00	32%	6,300.00	30%	5,735.00	0%	0.00
3410/3	Chalk(Steel Protect Coatings)	6,300.00	0%	0.00	100%	6,300.00	0%	0.00	0%	0.00
3420/3	Peel/Bub/Crack(Stil Protect Coal)	5,735.00	0%	0.00	<mark>0%</mark>	0.00	100%	5,735.00	0%	0.00
1000/3	Corrosion	390.00	0%	0.00	91%	353.00	9%	37.00	0%	0.00
1900/3	Distortion	143.00	0%	0.00	100%	143.00	0%	0.00	0%	0.00
109/3	Pre Opn Conc Girder/Beam	14,543.00	80%	11,650.00	9%	1,290.00	10%	1,468.00	1%	135.00
521/3	Conc Prol Coating	5,000.00	85%	4,250.00	0%	0.00	8%	375.00	8%	375.00
3510/3	Wear (Concrete Protect Coal)	750.00	0%	0.00	0%	0.00	50%	375.00	50%	375.00
1080/3	Detamination/Spail/Patched Area	1,221.00	0%	0.00	75%	910.00	25%	311.00	0%	0.00

RIDOT Inspection Report (v2.7.4cn)



Bridge Condition Poor

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Qty. St. 3 Elm/Env Description Total Qty % in 1 Qty. St. 1 % in 2 Qty. St. 2 % in 3 % in 4 Qty. St. 4 1090/3 Exposed Rebai 181.00 0% 0.00 3% 6.00 28% 50.00 69% 125.00 1100/3 Exposed Prestressing 25.00 0% 0.00 0% 0.00 10.00 60% 15.00 40% 1110/3 Cracking (PSC) 733.00 0% 0.00 1% 6.00 99% 727.00 0% 0.00 1120/3 Efflorescence/Rust Staining 730.00 365.00 0% 0.00 50% 365.00 50% 0% 0.00 7000/3 Damage 3.00 0% 0.00 100% 3.00 0% 0.00 0% 0.00 8368/3 Graffiti 200.00 100% 200.00 0% 0 00 0% 0.00 0% 0.00 110/3 Re Conc Opn Girder/Beam 33% 940.00 41% 1,188.00 702.00 50.00 24% 2% 2.880.00 Conc Prot Coating 521/3 14.800.00 100% 14.800.00 0% 0.00 0% 0 00 0% 0.00 1080/3 Delamination/Spall/Patched Area 0% 74% 808 00 0.00 600 00 26% 208 00 0% 0.00 Exposed Rebar 1090/3 0% 0.00 0% 100.00 0.00 50% 50.00 50% 50.00 1120/3 Efflorescence/Rust Staining 0% 450 00 0.00 67% 300.00 33% 150.00 0% 0.00 Cracking (RC and Other) 1130/3 0% 0.00 49% 0.00 582.00 288.00 51% 294.00 0% Re Conc Column 92.00 42% 39.00 17% 16.00 40% 37.00 0% 0.00 1080/3 Delamination/Spall/Patched Area 42.00 0% 0.00 38% 16.00 62% 26.00 0% 0.00 Efflorescence/Rust Staining 1120/3 5.00 0% 0.00 0% 0.00 100% 5.00 0% 0.00 Cracking (RC and Other) 1130/3 6.00 0% 0.00 0% 0.00 100% 6.00 0% 0.00 8368/3 Graffiti 300.00 0% 0.00 100% 300.00 0% 0.00 0% 0.00 210/3 Re Conc Pier Wall 657.00 290.00 204.00 1.151.00 57% 25% 8% 0% 0.00 521/3 Conc Prot Coating 25,200.00 100% 25,200.00 0% 0.00 0% 0.00 0% 0.00 1080/3 Delamination/Spall/Patched Area 184.00 0% 0.00 41% 75.00 59% 109.00 0% 0 00 1120/3 Efflorescence/Rust Staining 80.00 0% 0.00 50% 40.00 50% 40.00 0.00 0% 1130/3 Cracking (RC and Other) 115.00 0% 0.00 52% 60.00 48% 55.00 0% 0.00 6000/3 Scour 115.00 0.00 100% 0.00 0.00 0% 115.00 0% 0% 8368/3 Graffiti 400.00 0% 0.00 100% 400.00 0% 0.00 0% 0.00 **Re Conc Abutment** 215/3 78.00 34% 230.00 19% 44.00 47% 108.00 0% 0.00 521/3 Conc Prot Coating 2 300 00 100% 2 300 00 0% 0.00 0% 0.00 0% 0.00 1080/3 Delamination/Spall/Patched Area 0.00 28% 103.00 0% 29.00 72% 74.00 0% 0.00 1120/3 Efflorescence/Rust Staining 30.00 0% 50% 15 00 50% 15 00 0% 0.00 0 00 Cracking (RC and Other) 1130/3 0% 0.00 0.00 19 00 0% 0 00 100% 19 00 0% 8368/3 Graffit 50 00 0% 0.00 100% 50.00 0% 0.00 0% 0.00 220/3 Re Conc Pile Cap/Ftg 1.151.00 100% 1,150.00 1.00 0.00 0% 0.00 1130/3 Cracking (RC and Other) 0% 0.00 100% 1.00 0% 0.00 0% 0.00 1.00 234/3 Re Conc Pier Cap 388.00 13% 50.00 **B1%** 313.00 25.00 0% 0.00 1080/3 Delamination/Spall/Patched Area 310.00 0% 0.00 95% 293.00 5% 17.00 0% 0.00 1090/3 Exposed Rebar 0% 0.00 1.00 100% 1.00 0% 0.00 0% 0.00 1120/3 Efflorescence/Rust Staining 15.00 0% 0.00 47% 7.00 53% 8.00 0% 0.00 1130/3 Cracking (RC and Other) 12.00 0% 0.00 100% 12.00 0% 0.00 0% 0.00 Strip Seal Exp Joint 0.00 88.00 0.00 93.00 5.00)5% 2310/3 Leakage 0% 0.00 0% 5.00 100% 5.00 0.00 0% 0.00 2330/3 Seal Damage 10.00 0% 0.00 100% 10.00 0.00 0% 0.00 0% 2350/3 Debris Impaction 5.00 0% 0.00 100% 5.00 0% 0.00 0% 0.00 Metal Deterioration or Damage 2370/3 5 00 0% 0.00 0% 0.00 100% 5 00 0% 0.00 544.00 301/3 Pourable Joint Seal 1,151.00 44% 507.00 47% 85.00 15 00 2310/3 Leakage 0 00 344.00 0% 0 00 100% 344.00 0% 0.00 0% Seal Adhesion 2320/3 300.00 0% 0.00 67% 200.00 28% 85.00 5% 15.00 310/3 Elastomeric Bearing 136.00 190.00 401.00 34% 75.00 0.00 2220/3 Alignment 0% 0.00 0% 0.00 100% 4.00 0% 0.00 4.00 2230/3 Bulging, Splitting or Tearing 200.00 0% 0.00 75% 25% 50.00 0.00 150.00 0%



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in 1 Qty. St. 3 Elm/Env Description Total Qty % Qty. St. 1 % in 2 Qty. St. 2 % in 3 % in 4 Qty. St. 4 2240/3 Loss of Bearing Area 61.00 0% 0.00 66% 40.00 34% 21.00 0% 0.00 311/3 **Noveable Bearing** 0% 0.00 4% 7.00 4.00 0.00 11.00 36% 0% 515/3 Steel Protective Coating 0.00 132.00 0% 0% 0.00 33% 44.00 67% 88.00 3420 Peel/Bub/Crack(Stl Protect Coa 132.00 0.00 0% 0.00 44.00 679 88.00 0% 339 1000/3 Corrosion 0% 0.00 78% 7.00 22% 0.00 9.00 2.00 0% 2220/3 Alignment 0.00 1 00 0% 0% 0 00 100% 1 00 0% 0 00 2240/3 Loss of Bearing Area 1.00 0% 0.00 0% 100% 1.00 0.00 0.00 0% 313/3 0.00 Fixed Bearing 11.00 0% 8 00 3.00 0.00 73% 27% 0% 515/3 Steel Protective Coating 110.00 0% 0.00 0% 0.00 60% 66.00 40% 44.00 3420 Peel/Bub/Crack(Stl Protect Co. 110.00 0% 0.00 0% 0.00 60% 66.00 409 44.00 1000/3 Corrosion 11.00 0% 0.00 73% 8.00 27% 3.00 0% 0.00 321/3 Re Conc Approach Slab 2.352.00 0% 0.00 100% 2.352.00 0.00 0.00 510/3 Wearing Surfaces 2,352.00 57% 1,352.00 21% 500.00 21% 500.00 0% 0.00 3220/ Crack (Wearing Surface 2,352.0 57% 1,352.00 500.00 0.00 219 500.00 219 0% 331/3 Re Conc Bridge Railing 3,396.00 89% 411.00 0.00 3,808.00 11% 1.00 0% 1080/3 Delamination/Spall/Patched Area 10.00 0% 0.00 100% 10.00 0% 0.00 0% 0.00 1120/3 Efflorescence/Rust Staining 1 00 0% 0.00 0% 0.00 100% 1.00 0% 0.00 1130/3 Cracking (RC and Other) 351.00 0% 0.00 100% 0.00 351 00 0% 0% 0 00 7000/3 Damage 50.00 0% 0.00 100% 50.00 0% 0.00 0% 0.00 Scupper 0% 0.00 11% 3.00 20.00 15% 27.00 4.00 1000/3 Corrosion 0.00 0% 0% 4.00 4.00 0.00 0% 0.00 100% 8107/1 Steel Opn Girder/Beam EN 0.00 n ø 0.00 1009 110.00 0% 0.00 110.00 **n**% 515/1 Steel Protective Coating 1,615.00 0% 0.00 0% 0.00 38% 615.00 62% 1,000.00 3420/ Peel/Bub/Crack(Stl Protect Coal 1,615.00 0% 0.00 38% 615.00 629 1,000.00 0% 0.00 8213/3 R/C Return Wall 175.00 0% 0.00 86% 150.00 14% 25.00 0% 0.00 1080/3 De amination/Spall/Patched Area 44 00 0% 0.00 100% 44.00 0% 0 00 0% 0 00 1120/3 Efflorescence/Rust Staining 110.00 0% 0.00 77% 85.00 23% 25.00 0% 0.00 1130/3 Cracking (RC and Other) 21.00 0% 0.00 100% 21.00 0% 0.00 0% 0.00 8368/3 Graffiti 100.00 100% 100.00 0% 0% 0.00 0% 0.00 0.00 Backwall, All Types 8218/3 104.00 230.00 45% 35% 80.00 46.00 0% 0.00 1080/3 Delamination/Spall/Patched Area 0% 80.00 0.00 88% 70.00 13% 0% 0.00 10.00 1120/3 Efflorescence/Rust Staining 23 00 0% 0.00 43% 10 00 57% 13 00 0% 0.00 Cracking (RC and Other) 1130/3 23.00 0% 0 00 0% 0.00 100% 23 00 0% 0.00 8305/3 Asphaltic Joint Material 987.00 451.00 1.438.00 69% 0.00 0.00 2310/3 Leakage 430.00 0% 0 00 100% 430.00 0% 0.00 0% 0.00 2340/3 Seal Cracking 21.00 0% 0.00 100% 21.00 0% 0.00 0% 0.00 8335/3 Guardrail, Vehicular 700.00 550.00 21% 150.00 0.00 0.00 515/3 Steel Protective Coating 3,150.00 57% 1,800.00 0% 0.00 43% 1,350.00 0% 0.00 1000/3 Corrosion 100.00 0% 0.00 100% 100.00 0% 0.00 0% 0.00 1020/3 Connection 10.00 0% 0.00 100% 10.00 0% 0.00 0% 0.00 7000/3 Damage 40.00 0% 0.00 100% 40.00 0% 0.00 0% 0.00 8336/3 Conc Bridge Parapet 50% 350.00 6% 320.00 30.00 0.00 700.00 1080/3 D amination/Spall/Patched Area 0% 0.00 100.00 100% 100.00 0% 0.00 0.00 0% 1090/3 Exposed Rebar 0% 100.00 0.00 70% 70.00 30% 30.00 0% 0.00 1130/3 Cracking (RC and Other) 150.00 0% 0.00 100% 150.00 0% 0.00 0% 0.00 94% 940.00 30.00 Rip Rap 1.000.00 30.00 0% 0.00 4000/3 Settlement 60 00 0% 0.00 30.00 0.00 50% 50% 30.00 0% 8367/3 Slope Blocks 85% 595.00 0.00 15% 105.00 0.00 700.00 0% 0%



070001 Washington Bridge North

Bridge Condition Poor

Inspected By Inspector:

Last Inspection Date

07/22/2020

AECOM

Elm/Env	Description	Total Qty	% in 1	Qty. St. 1	% in 2	Qty. St. 2	% in 3	Qty. St. 3	% in 4	Qty. St. 4
8370/3	Steel Diaphragms	70.00	19%	13.00	51%	36.00	24%	17.00	6%	4.00
515/3	Steel Protective Coating	1,800.00	21%	378.00	63%	1,125.00	12%	207.00	5%	90.00
3410/3	Chalk(Steel Protect Coatings)	900.00	0%	0.00	100%	900.00	0%	0.00	0%	0.00
3420/3	Peel/Bub/Crack(Stl Protect Coat)	522.00	0%	0.00	43%	225.00	40%	207.00	17%	90.00
1000/3	Corrosion	55.00	0%	0.00	64%	35.00	29%	16.00	7%	4.00
1020/3	Connection	2.00	0%	0.00	50%	1.00	50%	1.00	0%	0.00
8371/3	Conc Diaphragms	221.00	10%	22.00	31%	68.00	57%	126.00	2%	5.00
1080/3	Delamination/Spall/Patched Area	65.00	0%	0.00	0%	0.00	100%	65.00	0%	0.00
1090/3	Exposed Rebar	12.00	0%	0.00	50%	6.00	8%	1.00	42%	5.00
1120/3	Efflorescence/Rust Staining	11.00	0%	0.00	55%	6.00	45%	5.00	0%	0.00
1130/3	Cracking (RC and Other)	111.00	0%	0.00	50%	56.00	50%	55.00	0%	0.00
8368/3	Graffiti	100.00	0%	0.00	100%	100.00	0%	0.00	0%	0.00
8398/1	Curb/sidewalks - Con	700.00	0%	0.00	100%	700.00	0%	0.00	0%	0.00
1080/1	Delamination/Spall/Patched Area	698.00	0%	0.00	100%	698.00	0%	0.00	0%	0.00
1120/1	Efflorescence/Rust Staining	1.00	0%	0.00	100%	1.00	0%	0.00	0%	0.00
1130/1	Cracking (RC and Other)	1.00	0%	0.00	100%	1.00	0%	0.00	0%	0.00

ELEMENT NOTES

ELEM	ELEMENT NAME	ENV	QUANTITY	UNITS	QTY CS 1	QTY CS 2	QTY CS 3	QTY CS 4
12	Re Concretie Deck	3	142,889.00	sq.ft	134,317.00	7,144.00	1,428.00	0.00

This element was not inspected per the scope of this special inspection. The following has been retained from the previous Routine Inspection Report dated 07/24/19: There is a reinforced concrete deck in Span #1 through #18. The top of the deck has a bituminous concrete wearing surface/overlay that was in varying stages of repair during the inspection (Photos 1-5, 42-49). The underside of the deck at the deck joints was in varying stages of re-construction during the inspection. Formwork remains in place throughout the bridge (Photos 26-36) and the seismic restrainer assemblies at the deck joints in Spans #1 though#6 and #8 through #14 typically have the restrainer rod removed (P hoto 97). The underside of the deck has areas of exposed rebar chairs throughout, areas of rust staining and efflorescence, random hairline cracking, random areas of damp concrete, random hollow areas and isolated spalls. The areas immediately surrounding drain pipes have heavy rust staining and efflorescence with intermittent hollow areas. The overhangs exhibit typical hairline transverse cracks with efflorescence and stalactites. See photos 92-1 05 and the attached file "070001 Elem 12 Defect Table.pdf" for further details.

Wea	aring Surffiaces	3	142,889.00	sq.ffi	134,317.00	7,144.00	1,428.00	0.00
Rou	tine Inspection Repo	rt dated 07/24/	2019:					
accu seal	bituminous concrete umulation along cons ed and unsealed lon ressed pavement wit	struction limits, gitudinal and tr	minor to mode ansverse crack	rate wheel I ks, scattered	ine rutting, rando 1 patches and	m		
	edges (Photos 46-4	9, 55-62).						
	edges (Photos 46-4 ELEMENT NAM	, ,	QUANTITY	UNITS	QTY	QTY	QTY	QTY
joint	5 (, ,	QUANTITY	UNITS	QTY CS 1	QTY CS 2	QTY CS 3	QTY CS 4

Routine Inspection Report dated 07/24/2019:

There are isolated minor potholes up to 3" deep and scattered depressed patches in the wearing surface. There is typical raveling or depressed areas up to 12" wide x 2" deep in the pavement along the joints (Photos 46-49, 55-62).



070001 Washington Bridge North

	d	OT		nspectio	оп кер	ort	Inspected E	By Inspector:	AEC
	Driven to ge	et you there	Bridge C	Condition	Poor		Last Inspec	tion Date	07/22/20
	ELEM	ELEMENT NAM	E ENV	QUANTITY	UNITS	QTY CS 1	QTY CS 2	QTY CS 3	QTY CS 4
		Crack (Wearing Sun Poutine Inspection Re		4,286.00 7/24/2019:	sq.ft	0.00	3,572.00	714.00	0.00
	th	here are isolated loc ne gore area in Span o the joints (Photos 5	s #15 throug	•		•			
080		minatton/Spall/Pattched		2,143.00	sq.ffi	0.00	1,786.00	357.00	0.00
	Rout	ine Inspection Repo	rt dated 07/24	4/2019:					
	See detai	photos 92-105 and tl ils.	he attached f	ile "070001 Ele	em 12 Defec	t Table.pdf" for fu	urther		
090	Expo	osed Rebar	3	2,143.00	sq.ffi	0.00	1,786.00	357.00	0.00
	Rout	ine Inspection Repor	rt dated 07/24	4/2019:					
	See detai	photos 92-105 and ti ils.	he attached f	ile "070001 Ele	em 12 Defec	t Table.pdf" for fu	urther		
120	Effior	rescence/Rustt Sttaining	3	2,143.00	sq.ffi	0.00	1,786.00	357.00	0.00
	Rout	ine Inspection Repo	rt dated 07/24	4/2019:					
	See detai	photos 92-105 and ti ils.	he attached f	ile "070001 Ele	em 12 Defec	t Table.pdf" for fu	urther		
130	Crack	king (RC and Otther)	3	2,143.00	sq.ffi	0.00	1,786.00	357.00	0.00
	Rout	ine Inspection Repo	rt dated 07/24	4/2019:					
	See detai	photos 92-105 and tl ils.	he attached f	ile "070001 Elé	em 12 Defec	t Table.pdf" for fu	urther		
				QUANTITY	UNITS	QTY	QTY	QTY	QTY
.EM	EL	EMENT NAME	ENV			CS 1	CS 2	CS 3	CS 4
LEM		EMENT NAME	ENV 3	7,336.00	sq.ft	CS 1 5,911.00	CS 2 1,150.00	CS 3 275.00	
	Re Con This ela Gano S of the ta areas o	c Top Flange ement defines the to treet off-ramp. The op flanges exhibit ty of heavy map cracks place. See photos 87	3 op flanges of top of the top ypical transv s with efflores	the reinforced o flanges has a erse hairline c scence, isolate	l concrete b a bituminou racks up to ed hollow ar	5,911.00 ox girders in Sp s concrete wear full width with e eas and spalls a	1,150.00 pans #1R, #2R, #3R ring surface/overlay offlorescence and r and ongoing repairs	275.00 and #5 of the 7. The undersides ust, scattered s with form work	CS 4
	Re Con This eld Gano S of the te areas o left in p details.	c Top Flange ement defines the to treet off-ramp. The op flanges exhibit ty of heavy map cracks place. See photos 87	3 op flanges of top of the top ypical transv s with efflores	the reinforced o flanges has a erse hairline c scence, isolate	l concrete b a bituminou racks up to ed hollow ar	5,911.00 ox girders in Sp s concrete wear full width with e eas and spalls a	1,150.00 pans #1R, #2R, #3R ring surface/overlay offlorescence and r and ongoing repairs	275.00 and #5 of the 7. The undersides ust, scattered s with form work	CS 4
	Re Con This ele Gano S of the to areas o left in p details.	c Top Flange ement defines the to street off-ramp. The op flanges exhibit ty of heavy map cracks place. See photos 87	3 op flanges of top of the top ypical transve with efflores 7, 89, 91, 93, 9 3	the reinforced o flanges has erse hairline c scence, isolate 96, 98 and the 7,336.00	d concrete b a bituminou cracks up to ed hollow ar attached file sq.ffi	5,911.00 ox girders in Sp s concrete wear full width with e eas and spalls a e "070001 Elem 7,336.00	1,150.00 pans #1R, #2R, #3R ring surface/overlay efflorescence and r and ongoing repairs 16 Defect Table.pdf	275.00 and #5 of the 7. The undersides ust, scattered s with form work " for further	CS 4 0.00
10	Re Con This ele Gano S of the to areas o left in p details. Weat	c Top Flange ement defines the to street off-ramp. The op flanges exhibit ty of heavy map cracks olace. See photos 87	3 op flanges of top of the top ypical transvo with efflores 7, 89, 91, 93, 9 3 a replaced prio	the reinforced o flanges has erse hairline c scence, isolate 96, 98 and the 7,336.00	d concrete b a bituminou cracks up to ed hollow ar attached file sq.ffi	5,911.00 ox girders in Sp s concrete wear full width with e eas and spalls a e "070001 Elem 7,336.00	1,150.00 pans #1R, #2R, #3R ring surface/overlay efflorescence and r and ongoing repairs 16 Defect Table.pdf	275.00 and #5 of the 7. The undersides ust, scattered s with form work " for further	CS 4 0.00
10	Re Con This eld Gano S of the to areas o left in p details. Weau The v Delau See	c Top Flange ement defines the to street off-ramp. The op flanges exhibit ty of heavy map cracks blace. See photos 87 ming Surffiaces wearing surface was	3 op flanges of top of the top ypical transve with efflores 7, 89, 91, 93, 9 3 s replaced prior Ar3	the reinforced o flanges has erse hairline c scence, isolate 36, 98 and the 7,336.00 or to 7/24/2019 200.00	d concrete b a bituminou cracks up to ed hollow ar attached file sq.ffi 9 Routine Ins sq.ffi	5,911.00 ox girders in Sp s concrete wear full width with e eas and spalls a e "070001 Elem 7,336.00 spection. 0.00	1,150.00 pans #1R, #2R, #3R ring surface/overlay efflorescence and r and ongoing repairs 16 Defect Table.pdf 0.00 200.00	275.00 and #5 of the y. The undersides ust, scattered s with form work " for further 0.00	CS 4 0.00
	Re Con This eld Gano S of the tr areas o left in p details. Weau The v Delau See for fu	c Top Flange ement defines the to breet off-ramp. The op flanges exhibit ty of heavy map cracks blace. See photos 87 ring Surffiaces wearing surface was minatton/Spall/Pattched photos 87, 89, 91, 93	3 op flanges of top of the top ypical transve with efflores 7, 89, 91, 93, 9 3 s replaced prior Ar3	the reinforced o flanges has erse hairline c scence, isolate 36, 98 and the 7,336.00 or to 7/24/2019 200.00	d concrete b a bituminou cracks up to ed hollow ar attached file sq.ffi 9 Routine Ins sq.ffi	5,911.00 ox girders in Sp s concrete wear full width with e eas and spalls a e "070001 Elem 7,336.00 spection. 0.00	1,150.00 pans #1R, #2R, #3R ring surface/overlay efflorescence and r and ongoing repairs 16 Defect Table.pdf 0.00 200.00	275.00 and #5 of the y. The undersides ust, scattered s with form work " for further 0.00	CS 4 0.00
10	Re Con This ele Gano S of the tr areas o left in p details. Wear The v Delar See f for fu	c Top Flange ement defines the to breet off-ramp. The op flanges exhibit ty of heavy map cracks blace. See photos 87 	3 op flanges of top of the top ypical transv with efflores 7, 89, 91, 93, 9 3 replaced prior Arr3 3, 96, 98 and 3	the reinforced o flanges has erse hairline c scence, isolate 96, 98 and the 7,336.00 or to 7/24/2019 200.00 the attached f 25.00	d concrete b a bituminou cracks up to ed hollow ar attached file sq.ffi d Routine Ins sq.ffi ile "070001 I sq.ffi	5,911.00 ox girders in Sp s concrete wear full width with e eas and spalls a e "070001 Elem 7,336.00 spection. 0.00 Elem 16 Defect T 0.00	1,150.00 Pans #1R, #2R, #3R ring surface/overlay efflorescence and r and ongoing repairs 16 Defect Table.pdf 0.00 200.00 Fable.pdf" 0.00	275.00 and #5 of the /. The undersides ust, scattered s with form work " for further 0.00 0.00	CS 4 0.00 0.00 0.00



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Inspected By

Bridge Condition Poor

Inspector: Last Inspection Date

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See photos 87, 89, 91, 93, 96, 98 and the attached file "070001 Elem 16 Defect Table.pdf"
for further details.

1130	Cracking (RC and Otther)	3	200.00	sq.ffi	0.00	200.00	0.00	0.00
	See photos 87, 89, 91, for further details	93, 96, 98 a	nd the attached f	ile "070001 I	Elem 16 Defect	Table.pdf"		
ELEM	ELEMENT NAME	ENV	QUANTITY	UNITS	QTY	QTY	QTY	QTY
ELEM	ELEMENT NAME	ENV	QUANTITY	UNITS	QTY CS 1	QTY CS 2	QTY CS 3	QTY CS 4

	There are reinforced con Street off-ramp. The box as the main bridge struct assemblies and cables ar typical full height vertica gauges in place to monit See the attached file "070 to 6" deep at Pier #2R (Pl details of scattered mino scattered transverse hair photos 22-25, 77-82 and t	girder cells ture. Span b t Pier #2R en l/diagonal h or the move 0001 Elem 1 hotos 90, 92 r defects an fline cracks	are lettered 'A' th ays are numbere chibit typical rus airline cracks, be ment of these cr 05 Defect 1130 T , 94). See the att d notes. The und with efflorescen	hrough 'C' f ed 1 through t with light oth sealed a acks and at able.pdf" fo ached file " dersides of ce and rust	from south to no n 3 from west to corrosion (Photo and unsealed (Ph t the time of insp or further details. 070001 Elem 105 the bottom flang staining and iso	rth to maintain the east. The seismic os 87, 88). The intentent noto 84). There are ection no movement There is typical por Defect Table.pdf" res have random resonant	same orientation restrainer rior webs exhibit numerous ent was detected. onding water up for further epair patches, and spalls. See	
1080	Delaminatton/Spall/Pattch		100.00	ffi	0.00	80 00	20.00	0.00
	See photos 76-98 and 1 Elem 105 Defect Tabl details.				•	-		
1090	Exposed Rebar	3	5.00	ffi	0.00	0.00	5.00	0.00
	See photos 76-98 and 1 Elem 105 Defect Tabl details.				•	-		
1120	Effiorescence/Rustt Sttainin	ng 3	244.00	ffi	0.00	122.00	122.00	0.00
	See photos 76-98 and 1 Elem 105 Defect Tabl details.							
1130	Cracking (RC and Otther)	3	495.00	ffi	0.00	303.00	192.00	0.00
	See photos 76-98 and 1 Elem 105 Defect Tabl details.							
LEM	ELEMENT NAME	ENV	QUANTITY	UNITS	QTY CS 1	QTY CS 2	QTY CS 3	QTY CS 4
	Stieel Opn Girder/Beam	3	1,320.00	ft	787.00	496.00	37.00	0.00

515

0.00

5,735.00

Stteel Prottecttve Coattng

3

19,385.00

sq.ffi

7,350.00

6,300.00

070001

	RIDOT Bri Inspection F			•		v	Washington Bridge North			
		OT			spectio	птерс	// (Inspected B	y	AECON
									Inspector:	
	Driven to	get you there	Brid	lge C	ondition F	oor		Last Inspec	tion Date	07/22/2020
	are	e fascia sides of as have light to d the attached fil	moderate rus	t with u	up to heavy rus	t at girder er	nds. See photos	•		
	ELEM	ELEMENT		ENV	QUANTITY	UNITS	QTY	QTY	QTY	QTY
							CS 1	CS 2	CS 3	CS 4
	3410	Chalk(Steel P	rotect Co 3		6,300.00	sq.ft	0.00	6,300.00	0.00	0.00
		See photos 37-	41 and the att	ached	file "070001 El	em 107 Defe	ect Table.pdf" fo	or further details.		
	ELEM	ELEMENT		ENV	QUANTITY	UNITS	QTY	QTY	QTY	QTY
							CS 1	CS 2	CS 3	CS 4
	3420	Peel/Bub/Crac	ck(Stl Prc 3		5,735.00	sq.ft	0.00	0.00	5,735.00	0.00
		See photos 37-4	41 and the att	ached	file "070001 El	em 107 Defe	ect Table.pdf" fo	or further details.		
1000	Co	rrosion	3		390.00	ffi	0.00	353.00	37.00	0.00
		e photos 37-41, ails.	108 and the a	attache	d file "070001	Elem 107 De	efect Table.pdf"	for further		
1900	Dis	sttortton	3		143.00	ffi	0.00	143.00	0.00	0.00
	The).	e bottom flanges	s exhibit typica	al 1/8" v	vertical distortion	on at the sec	ction transitions	(Photo 39		
		der 'A' bottom fla he girder (top of	0		•			rotation		

DIDAT Dridaa

ELEM	ELEMENT NAME	ENV	QUANTITY	UNITS	QTY CS 1	QTY CS 2	QTY CS 3	QTY CS 4
109	Pre Opn Conc Girder/Beam	3	14,543.00	ft	11,650.00	1,290.00	1,468.00	135.00

The prestressed concrete girders in Spans #1 through #6 and #8 through #14 consist of variable depth post-tension ed cantilevered girder sections over the piers with corbels at the end. The cantilevered girder sections support prestressed concrete drop-in mid-span sections (Photos 16-18). The prestressed concrete I-girders in Spans #15 through #18 are simply supported between the substructure units (Photo 21). Rehabilitation construction is on-going and there are multiple defects that have been repaired or are in the process of being repaired. The drop-in girders exhibit typical shear cracks at dapped ends, scattered cracked, hollow and spalled areas at dapped ends and bottom flanges undersides with exposed stirrups and prestressing strands, scattered cracked, hollow and spalled areas over the bearings with fully exposed stirrups and reduced bearing areas. See photos 42-54 and the attached files "070001 Elem 109 Shear Crack Table.pdf" and "070001 Elem 109 Defect Table.pdf" for further details. The corbels exhibit typical cracked, hollow and spalled areas with exposed post tensioned anchor plates on the drop-in span sides throughout. The other faces and undersides exhibit isolated cracks, hollow areas and minor spalls. See photos 55-59 and the attached file "070001 Elem 109 Defect Table.pdf" for further details. The cantilever girders exhibit typical hairline diagonal cracks along the post-tensioned cable lines, some sealed and unsealed, isolated vertical cracks and hollow area over the pier columns and typical hollow/spalled post-tensioned anchor blocks on the undersides. Other remaining areas exhibit random minor cracked, hollow and spalled areas. The cantilever ends in Span #7 at Pier #6 and Pier #7 (accessed via the catwalks on the interior walls of the piers) exhibit typical hollow areas/spalls up to full height with fully exposed and debonded stirrups and reduced bearing areas. See photos 66-75 and the attached file "070001 Elem 109 Defect Table.pdf" for further details. The I-girders in Spans #15 through #18 have scattered hairline cracking with efflorescence, hollow areas, spalls and exposed prestressing strands at girder ends, with more severe spalling and exposed stirrups on the back faces beyond the bearings. There are isolated hollow areas and spalls along bottom flange undersides. See photos 60-65 and the attached file " 070001 Elem 109 Defect Table.pdf" for further details.

 521
 Conc Prott Coattng
 3
 5,000.00
 sq.ffi
 4,250.00
 0.00
 375.00
 375.00

The drop-in girder dapped ends are coated with a protective sealant which has scattered peeling and cracking throughout (Photos 42-75).



070001 Washington Bridge North

1		OT	In	spectio	n Repo	ort	Inspected E		AEC
	Driven to g	et you there	Bridae C	ondition	Poor		Last Inspec	Inspector:	07/22/2
	ELEM	ELEMENT NAM	•		UNITS	QTY CS 1	QTY CS 2	QTY CS 3	QTY CS 4
		Wear (Concrete Pro ee 521 - Concrete F		750.00 ting notes.	sq.ft	0.00	0.00	375.00	375.00
80	Dela	minatton/Spall/Pattche	d Ar3	1,221.00	ffi	0.00	910.00	311.00	0.00
		photos 42-75 and th 001 Elem 109 Defe				r Crack Table.po	df" and		
090	Expo	sed Rebar	3	181.00	ffi	0.00	6.00	50.00	125.00
		photos 42-75 and th 001 Elem 109 Defe				r Crack Table.po	df" and		
100	Expo	sed Presttressing	3	25.00	ffi	0.00	0.00	15.00	10.00
		photos 42-75 and th 001 Elem 109 Defe				r Crack Table.po	df" and		
110	Cracl	king (PSC)	3	733.00	ffi	0.00	6.00	727.00	0.00
		photos 42-75 and th 001 Elem 109 Defe				r Crack Table.po	df" and		
120	Effio	rescence/Rustt Sttaining	g 3	730.00	ffi	0.00	365.00	365.00	0.00
		photos 42-75 and th 001 Elem 109 Defe				r Crack Table.po	df" and		
000	Dam	age	3	3.00	ffi	0.00	3.00	0.00	0.00
	lanes - Spa	prestressed concret s in the following loc an #16 Girder 'E' ea	cations: ist of midspan:	3' long x up to	1⁄4" deep sc	-	ver travel		
368	- Spa Graff	an #18 All girders: N	3	200.00	ffi	200.00	0.00	0.00	0.00
		drop-in girder ends							0.00
LEM	EL	EMENT NAME	ENV	QUANTITY	UNITS	QTY CS 1	QTY CS 2	QTY CS 3	QTY CS 4
	Po Con	o Onn Cindon /Room	·						
LEM					UNITS				
)	This eld R (Phot cantiley Rehabi of being hairline	c Opn Girder/Beam ement defines reinf tos 5-8, 29-36). The ver sections suppo litation constructio g repaired. The arc to medium horizo eys with several thr	arches consis ort the drop-in on is on-going hes exhibit typ ntal cracks at	st of cantileve sections with and there are pical vertical a the shiplap jo	red sections concrete ke multiple def nd transver ints, scatter	at the piers an ys at shiplap jo ects that have se hairline crac ed hollow areas	d drop-in midspan ints with elastome been repaired or ar ks in the midspan s and spalls above	sections. The ric bearing pads. re in the process sections, typical and below the	5
0	This eld R (Phot cantilev Rehabi of bein hairline joint ke and spa	ement defines reinf tos 5-8, 29-36). The ver sections suppo litation constructio g repaired. The arc	forced concret arches consis rt the drop-in n is on-going hes exhibit typ ntal cracks at ough holes, ex	te fascia arche st of cantileve sections with and there are pical vertical a the shiplap jo xposed and de	es in Spans a red sections concrete ke multiple def nd transver ints, scatter abonded stir	#1 through #6, # at the piers an ys at shiplap jo ects that have se hairline crac ed hollow areas rups and rebar	#8 through #13 and d drop-in midspan ints with elastome been repaired or ar ks in the midspan s and spalls above s, and scattered cr	I #1R through #3 sections. The ric bearing pads. re in the process sections, typical and below the acked, hollow	50.00
	This eld R (Phot cantiley Rehabi of beiny hairline joint ke and spa for furt	ement defines reinf tos 5-8, 29-36). The ver sections suppo litation constructio g repaired. The arc e to medium horizo eys with several thr alled areas on the b	forced concret arches consis rt the drop-in n is on-going hes exhibit typ ntal cracks at ough holes, ex	te fascia arche st of cantileve sections with and there are pical vertical a the shiplap jo xposed and de	es in Spans a red sections concrete ke multiple def nd transver ints, scatter abonded stir	#1 through #6, # at the piers an ys at shiplap jo ects that have se hairline crac ed hollow areas rups and rebar	#8 through #13 and d drop-in midspan ints with elastome been repaired or ar ks in the midspan s and spalls above s, and scattered cr	I #1R through #3 sections. The ric bearing pads. re in the process sections, typical and below the acked, hollow	0.00
.0	This eld R (Phot cantilex Rehabi of bein hairline joint ke and spa for furt	ement defines reinf tos 5-8, 29-36). The ver sections suppo litation constructio g repaired. The arc to medium horizon sys with several thr alled areas on the t her details.	forced concret arches consis ort the drop-in on is on-going hes exhibit typ ntal cracks at ough holes, ep bottom flanges 3 and bottom flar ee photos 29-3	te fascia arches st of cantileve sections with and there are pical vertical a the shiplap jo xposed and de s. See photos 14,800.00 nges are partia	es in Spans a red sections concrete ke multiple def nd transver ints, scatter ebonded stin 29-36 and th sq.ffi illy coated w	#1 through #6, # at the piers an ys at shiplap jo fects that have I se hairline crac ed hollow areas rrups and rebar ne attached file 14,800.00 ith a new protect	#8 through #13 and d drop-in midspan ints with elastome been repaired or ar ks in the midspan s and spalls above s, and scattered cr "070001 Elem 110 l 0.00	I #1R through #3 sections. The ric bearing pads. re in the process sections, typical and below the acked, hollow Defect Table.pdf"	



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See photos 29-36 and the attached file "070001 Elem 110 Defect Table.pdf" for further
details.

Bridge Condition Poor

120	Delaminatton/Spall/Pattche See photos 15-17, 21, 1 pdf" for further details. Effiorescence/Rustt Sttaining See photos 15-17, 21, 1 pdf" for further details. Cracking (RC and Otther) See photos 15-17, 21, 1 pdf" for further details. Graffitt The Pier #3 columns hav	16, 120-122 g 3 16, 120-122 3 16, 120-122 3	5.00 2 and the attache 6.00 2 and the attache 300.00	each ed file "07000 each ed file "07000 each ed file "07000 each	0.00 11 Elem 205 Defe 0.00 11 Elem 205 Defe 0.00 11 Elem 205 Defe 0.00	16 00 ect Table. 0.00 ect Table. 0.00	ails. 26.00 5.00 6.00 0.00 0.00 QTY CS 3	0.00 0.00 0.00 0.00 QTY CS 4
1120	See photos 15-17, 21, 1 pdf" for further details. Effiorescence/Rustt Sttaining See photos 15-17, 21, 1 pdf" for further details. Cracking (RC and Otther) See photos 15-17, 21, 1 pdf" for further details. Graffitt	16, 120-122 g 3 16, 120-122 3 16, 120-122 3	2 and the attache 5.00 2 and the attache 6.00 2 and the attache 300.00	each ed file "07000 each ed file "07000 each ed file "07000 each	0.00 11 Elem 205 Defe 0.00 11 Elem 205 Defe 0.00 11 Elem 205 Defe 0.00	16 00 ect Table. 0.00 ect Table. 0.00 ect Table.	26.00	0.00
1120	See photos 15-17, 21, 1 pdf" for further details. Effiorescence/Rustt Sttaining See photos 15-17, 21, 1 pdf" for further details. Cracking (RC and Otther) See photos 15-17, 21, 1 pdf" for further details. Graffitt	16, 120-122 g 3 16, 120-122 3 16, 120-122 3	2 and the attache 5.00 2 and the attache 6.00 2 and the attache 300.00	each ed file "07000 each ed file "07000 each ed file "07000 each	0.00 11 Elem 205 Defe 0.00 11 Elem 205 Defe 0.00 11 Elem 205 Defe 0.00	16 00 ect Table. 0.00 ect Table. 0.00 ect Table.	26.00	0.00
120	See photos 15-17, 21, 1 pdf" for further details. Effiorescence/Rustt Sttaining See photos 15-17, 21, 1 pdf" for further details. Cracking (RC and Otther) See photos 15-17, 21, 1	16, 120-122 g 3 16, 120-122 3	2 and the attache 5.00 2 and the attache 6.00	each ed file "07000 each ed file "07000 each	0.00 11 Elem 205 Defe 0.00 11 Elem 205 Defe 0.00	16 00 ect Table. 0.00 ect Table. 0.00	26.00	0.00
120	See photos 15-17, 21, 1 pdf" for further details. Effiorescence/Rustt Sttaining See photos 15-17, 21, 1 pdf" for further details. Cracking (RC and Otther)	16, 120-122 g 3 16, 120-122 3	2 and the attache 5.00 2 and the attache 6.00	each ed file "07000 each ed file "07000 each	0.00 11 Elem 205 Defe 0.00 11 Elem 205 Defe 0.00	16 00 ect Table. 0.00 ect Table. 0.00	26.00	0.00
	See photos 15-17, 21, 1 pdf" for further details. Effiorescence/Rustt Sttaining See photos 15-17, 21, 1	16, 120-122 g 3	2 and the attache	each ed file "07000 each	0.00 11 Elem 205 Defe 0.00	16 00 ect Table. 0.00	26.00	
	See photos 15-17, 21, 1 pdf" for further details.	16, 120-122	2 and the attache	each ed file "07000	0.00 1 Elem 205 Defe	16 00 ect Table.	26.00	
1080	See photos 15-17, 21, 1			each	0.00	16 00		0.00
1080	Delaminatton/Spall/Pattche	d Ar3	42.00		-			0.00
					5 Delect Table.p	odf" for further deta	ails.	
05	There are reinforced conc through #17 that support columns exhibit isolated h top of the columns exhibit edges of steel bearing pla rust staining throughout v photos 15-17, 116, 120-122	rete colum the reinforc nairline vert t typical sca ttes. The pie with isolated	ns at Piers 1# th ed concrete pier cical and map cra attered hollow a er cap columns e d hairline map co	r caps (Phot acks, hollow reas/spalls u exhibit typic racks, efflor	nat support the o os 15-17, 21, 110 areas and spall up to full width x al scattered sea escence, hollow	cantilever girders a 6, 120-122). The ca Is (Photo 122). The t full height x 2" de led/unsealed vertio	and at Piers #14 ntilever girder pedestals at the ep with exposed cal cracks and	0.00
15	Re Conc Column	3	92.00	each	39.00	16.00	37.00	0.00
LEM	ELEMENT NAME	ENV	QUANTITY	UNITS	QTY CS 1	QTY CS 2	QTY CS 3	QTY CS 4
	See photos 29-36 and th details.	ne attached	file "070001 Eler	m 110 Defec	t Table.pdf" for fu	urther		
1130	Cracking (RC and Otther)	3	582.00	ffi	0.00	288.00	294.00	0.00
	See photos 29-36 and the details.	ne attached	file "070001 Eler	m 110 Defec	t Table.pdf" for fu	urther		
1120	Effiorescence/Rustt Sttaining	g 3	450.00	ffi	0.00	300.00	150.00	0.00
120	See photos 29-36 and the details.	ne attached	file "070001 Elei	m 110 Defec	t Table.pdf" for fu	urther		
126			100.00	ffi	0.00	0.00	50.00	50.00



RIDOT Bridge Inspection Report

070001 Washington Bridge North

Inspector:

Inspected By

AECOM

L	Driven to get you there	Bridge	Condition	Poor		Last Inspec	tion Date	07/22/2020
	There are reinforced compier wall of Pier #6, the wwalls providing architect). The east pier wall of Pier (through cantilever suppedestals can be accessed of report (Photos 14 superstructure (Photos 14 superstructure (Photos 7) piers that extend from the protective coating in cracks re-opening (Photot 123). efflorescence and rust so 210 Defect Table.pdf" for	vest pier wall tural (stone f er #6 and the oort pedestal ed via the ca 2 - 144). Pier 15, 23 - 25). T te coping at t n most locati os 16 - 18, 12 The pylons r taining. See	of Pier #7 and F açade) and prote west pier wall of s) and the steel of twalks on the in walls #1R throu here are reinford the base of the b ons and all piers (5, 135, 136). Sca emain uncoated photos 16 - 20, 2	Piers #1R thr ective effects of Pier #7 su girders in Sp terior portio gh #3R supp ced concrete ridge railing s have seale attered crack	ough #3R are no s to the pier colu pport the cantile oan #7 (Photos 1 ns of Pier #6 and oort the Gano Sta opylons/ walls a s (Photos 5, 7, 1 d vertical and m as through the pi typical scattered	on-structural and a imns (Photos 16 - ever girder ends in 9, 20). The cantiled d Pier #7; see insp reet off-ramp box t the north and so 40). The pier walls ap cracks through ier wall stone faca d hairline cracking	act as curtain 18, 125, 135, 136 Spans #6 and #8 ver girder ection notes at girder uth ends of the s on land have a out with isolated des remain	
521	Conc Prott Coattng	3	25,200.00	sq.ffi	25,200.00	0.00	0.00	0.00
	The pier walls on land , 135-140 and the atta deterioration.		-					
1080	Delaminatton/Spall/Pattch	ied Ar3	184.00	ffi	0.00	75 00	109.00	0.00
	See photos 16 - 20, 23 Defect Table.pdf" for de			the attached	file "070001 Ele	m 210		
1120	Effiorescence/Rustt Sttaini	ng 3	80.00	ffi	0.00	40 00	40.00	0.00
	See photos 16 - 20, 23 Defect Table.pdf [*] for de			he attached	file "070001 Eler	n 210		
1130	Cracking (RC and Otther)	3	115.00	ffi	0.00	60 00	55.00	0.00
	See photos 16 - 20, 23 Defect Table.pdf" for de			he attached	file "070001 Eler	n 210		
6000	Scour	3	115.00	ffi	0.00	115.00	0.00	0.00
	2017 Underwater Insp Since the 2013 Underv deep (Pier #8) and are	vater Inspecti				p to 3.4'		
8368	Graffitt	3	400.00	ffi	0.00	400.00	0.00	0.00
	The pier walls on land).	exhibit isolate	ed moderate to h	eavy graffiti	(Photos 16, 121,	135 - 137,		
ELEM	ELEMENT NAME	ENV	QUANTITY	UNITS	QTY CS 1	QTY CS 2	QTY CS 3	QTY CS 4
215	Re Conc Abutimenti	3	230.00	ft	78.00	44.00	108.00	0.00

There are reinforced concrete abutments at each end of the main structure (West Abutment #1 & East Abutment #2) and at the end of the Gano Street off-ramp (West Abutment #1R). The abutments all have new protective coatings. West Abutment #1 is a stub abutment that is hidden by backfill beyond a retaining wall (Photo 111). There is severe accumulation of pigeon debris and nesting pigeons behind the wall up to the top of the columns preventing the inspection of the stub abutment stem (Photo 112). The retaining wall exhibits scattered hairline cracking. East Abutment #2 is a full height abutment with an electrical utility room built into the abutment in Bays 'H' and 'I' (Photo 113). See inspection notes for electrical room notes. The abutment exhibits scattered hairline cracks, hollow areas and spalls with typical debris accumulation/pigeon nesting on the beam seat (Photos 114, 115). West Abutment #1R is a semi-stub abutment that sits on the river embankment with slope protection blocks in front (Photo 22). The abutment exhibits scattered efflorescence and rust staining and an isolated spall. See photos 22, 111-115 and the attached file "070001 Elem 215 Defect Table.pdf" for details of deterioration.

			RIDOT Inspectio	-		Wa Inspected By	shington	07000 [,] Bridge Nortl AECOI
	Driven to get you there	Bridge	Condition	Poor		Last Inspectio	Inspector: n Date	07/22/2020
521	Conc Prott Coattng	3	2,300.00	sq.ffi	2,300.00	0.00	0.00	0.00
	The abutments all have 070001 Elem 215 Defec	protective c	oatings. See pho	otos 22, 111-	,			
1080	Delaminatton/Spall/Pattched	d Ar3	103.00	ffi	0.00	29 00	74.00	0.00
	See photos 22, 111-115 details of deterioration.	and the atta	ached file "07000	1 Elem 215	Defect Table.pdf" fo	Dr		
1120	Effiorescence/Rustt Sttaining	g 3	30.00	ffi	0.00	15 00	15.00	0.00
	See photos 22, 111-115 details of deterioration.	and the atta	ached file "07000)1 Elem 215	Defect Table.pdf" f	for		
1130	Cracking (RC and Otther)	3	19.00	ffi	0.00	0.00	19.00	0.00
	See photos 22, 111-115 details of deterioration.	and the atta	ached file "07000	1 Elem 215	Defect Table.pdf" fo	or		
8368	Graffitt	3	50.00	ffi	0.00	50 00	0.00	0.00
	Abutment #1R has a 5'-0)" high x 10'	-0" wide area of	light graffiti (Photo 22).			
ELEM	ELEMENT NAME	END /			QTY	QTY	OTY	QTY
		ENV	QUANTITY	UNITS			QTY CS 3	CS 4
	Re Conc Pile Cap/Ftig 2017 Underwater Inspectio 10" wide to 18" wide and a	3 on: The exp	1,151.00 osed pile caps s	ft step out fron	CS 1 1,150.00 n the face of the p	CS 2 1.00 ier stems at varying	CS 3 0.00 widths from	
	Re Conc Pile Cap/Ftig 2017 Underwater Inspectio	3 on: The exp are exposed R (Gano Str	1,151.00 osed pile caps s d up to full-heigh reet Ramp). Piers	ft step out fron it with varyii s #3R, #5 an	CS 1 1,150.00 In the face of the p ing measurements d #9 exhibit expos	CS 2 1.00 ier stems at varying from 2' (full-height) sed concrete tremie	CS 3 0.00 widths from at Pier #5 to	CS 4 0.00
220	Re Conc Pile Cap/Ftig 2017 Underwater Inspectio 10" wide to 18" wide and a 9.0' (full-height) at Pier #31 maximum vertical exposu	3 on: The exposed R (Gano Str re of 3.5' hig 3 stion:	1,151.00 osed pile caps s I up to full-heigh reet Ramp). Piers gh. There is no c	ft step out from it with varyin s #3R, #5 an observed un ffi	CS 1 1,150.00 In the face of the p ing measurements d #9 exhibit expos dermining at any o	CS 2 1.00 ier stems at varying from 2' (full-height) sed concrete tremie of the piers. 1.00	CS 3 0.00 widths from at Pier #5 to seals up to a	CS 4 0.00
220	Re Conc Pile Cap/Ftig 2017 Underwater Inspectio 10" wide to 18" wide and a 9.0' (full-height) at Pier #31 maximum vertical exposu Cracking (RC and Otther) 2017 Underwater Inspec	3 on: The exposed R (Gano Str re of 3.5' hig 3 stion:	1,151.00 osed pile caps s I up to full-heigh reet Ramp). Piers gh. There is no c	ft step out from it with varyin s #3R, #5 an observed un ffi	CS 1 1,150.00 In the face of the p ing measurements d #9 exhibit expos dermining at any o	CS 2 1.00 ier stems at varying from 2' (full-height) sed concrete tremie of the piers. 1.00	CS 3 0.00 widths from at Pier #5 to seals up to a	CS 4 0.00
220 1130 ELEM	Re Conc Pile Cap/Ftig 2017 Underwater Inspectio 10" wide to 18" wide and a 9.0' (full-height) at Pier #31 maximum vertical exposu Cracking (RC and Otther) 2017 Underwater Inspec Pier #3R pile cap has a c	3 on: The exposed R (Gano Str re of 3.5' hig 3 stion: crack 6' high	1,151.00 oosed pile caps s d up to full-heigh reet Ramp). Piers gh. There is no c 1.00 n x 3/16" wide ex	ft step out from it with varyin s #3R, #5 an observed un ffi tending from	CS 1 1,150.00 In the face of the p ing measurements d #9 exhibit expose dermining at any o 0.00 0.00 In the top of the pile QTY	CS 2 1.00 ier stems at varying from 2' (full-height) sed concrete tremie of the piers. 1.00 cap. QTY	CS 3 0.00 widths from at Pier #5 to seals up to a 0.00 QTY	CS 4 0.00 0.00 QTY
220 	Re Conc Pile Cap/Ftig 2017 Underwater Inspectio 10" wide to 18" wide and a 9.0' (full-height) at Pier #31 maximum vertical exposur Cracking (RC and Otther) 2017 Underwater Inspect Pier #3R pile cap has a d ELEMENT NAME	3 on: The exposed R (Gano Str re of 3.5' high 3 stion: crack 6' high ENV 3 rete caps at tion materia	1,151.00 oosed pile caps s I up to full-heigh reet Ramp). Piers gh. There is no c 1.00 n x 3/16" wide ex QUANTITY 388.00 t Piers #14 throu als throughout (F	ft step out from it with varying s #3R, #5 an observed un ffi tending from UNITS ft ligh #17. The Photos 21, 1	CS 1 1,150.00 In the face of the p ing measurements d #9 exhibit expose dermining at any of 0.00 In the top of the pile QTY CS 1 50.00 Caps were recent 16). The caps and	CS 2 1.00 ier stems at varying from 2' (full-height) sed concrete tremie of the piers. 1.00 cap. QTY CS 2 313.00 ly repaired and are pedestals exhibit is	CS 3 0.00 widths from at Pier #5 to seals up to a 0.00 QTY CS 3 25.00 covered with solated	CS 4 0.00 0.00 0.00 QTY CS 4 0.00
220 1130 ELEM	Re Conc Pile Cap/Ftig 2017 Underwater Inspection 10" wide to 18" wide and a 9.0' (full-height) at Pier #31 maximum vertical exposure Cracking (RC and Otther) 2017 Underwater Inspect Pier #3R pile cap has a of ELEMENT NAME Re Conc Pier Cap There are reinforced concount remaining chloride extract hairline cracks, hollow are	3 on: The exposed R (Gano Str re of 3.5' high 3 stion: crack 6' high ENV 3 rete caps at tion materia a and spall	1,151.00 oosed pile caps s I up to full-heigh reet Ramp). Piers gh. There is no c 1.00 n x 3/16" wide ex QUANTITY 388.00 t Piers #14 throu als throughout (F	ft step out from it with varying s #3R, #5 an observed un ffi tending from UNITS ft ligh #17. The Photos 21, 1	CS 1 1,150.00 In the face of the p ing measurements d #9 exhibit expose dermining at any of 0.00 In the top of the pile QTY CS 1 50.00 Caps were recent 16). The caps and	CS 2 1.00 ier stems at varying from 2' (full-height) sed concrete tremie of the piers. 1.00 cap. QTY CS 2 313.00 ly repaired and are pedestals exhibit is	CS 3 0.00 widths from at Pier #5 to seals up to a 0.00 QTY CS 3 25.00 covered with solated	CS 4 0.00 0.00 0.00 QTY CS 4 0.00
220 1130 ELEM 234	Re Conc Pile Cap/Ftig 2017 Underwater Inspection 10" wide to 18" wide and a 9.0' (full-height) at Pier #31 maximum vertical exposure Cracking (RC and Otther) 2017 Underwater Inspect Pier #3R pile cap has a construct Pier #3R pile cap has a construct Re Conc Pier Cap There are reinforced concorremaining chloride extract hairline cracks, hollow are pdf" for further details.	3 on: The exposed R (Gano Str re of 3.5' high 3 ction: crack 6' high ENV 3 rete caps at tion materia ba and spall d Ar3	1,151.00 oosed pile caps s I up to full-heigh reet Ramp). Piers gh. There is no c 1.00 n x 3/16" wide ex QUANTITY 388.00 t Piers #14 throu als throughout (F s. See photos 24 310.00	ft step out from it with varyin s #3R, #5 an observed un ffi tending from UNITS ft uNITS ft igh #17. The Photos 21, 1: 1, 116-119 ar ffi	CS 1 1,150.00 In the face of the p ing measurements d #9 exhibit expose dermining at any of 0.00 In the top of the pile QTY CS 1 50.00 Caps were recent 16). The caps and ind the attached file 0.00	CS 2 1.00 ier stems at varying from 2' (full-height) sed concrete tremie of the piers. 1.00 cap. QTY CS 2 313.00 ly repaired and are pedestals exhibit is a "070001 Elem 234 293.00	CS 3 0.00 widths from at Pier #5 to seals up to a 0.00 QTY CS 3 25.00 covered with solated Defect Table	CS 4 0.00 0.00 0.00 QTY CS 4 0.00
220 1130 ELEM 234	Re Conc Pile Cap/Ftig 2017 Underwater Inspection 10" wide to 18" wide and a 9.0' (full-height) at Pier #31 maximum vertical exposure Cracking (RC and Otther) 2017 Underwater Inspect Pier #3R pile cap has a description ELEMENT NAME Re Conc Pier Cap There are reinforced concer remaining chloride extract hairline cracks, hollow are pdf" for further details. Delaminatton/Spall/Pattcherer See photos 21, 116-119	3 on: The exposed R (Gano Str re of 3.5' high 3 ction: crack 6' high ENV 3 rete caps at tion materia ba and spall d Ar3	1,151.00 oosed pile caps s I up to full-heigh reet Ramp). Piers gh. There is no c 1.00 n x 3/16" wide ex QUANTITY 388.00 t Piers #14 throu als throughout (F s. See photos 24 310.00	ft step out from it with varyin s #3R, #5 an observed un ffi tending from UNITS ft uNITS ft igh #17. The Photos 21, 1: 1, 116-119 ar ffi	CS 1 1,150.00 In the face of the p ing measurements d #9 exhibit expose dermining at any of 0.00 In the top of the pile QTY CS 1 50.00 Caps were recent 16). The caps and ind the attached file 0.00	CS 2 1.00 ier stems at varying from 2' (full-height) sed concrete tremie of the piers. 1.00 cap. QTY CS 2 313.00 ly repaired and are pedestals exhibit is a "070001 Elem 234 293.00	CS 3 0.00 widths from at Pier #5 to seals up to a 0.00 QTY CS 3 25.00 covered with solated Defect Table	CS 4 0.00 0.00 0.00 QTY CS 4 0.00
220 1130 ELEM 234	Re Conc Pile Cap/Ftig 2017 Underwater Inspection 10" wide to 18" wide and a 9.0' (full-height) at Pier #31 maximum vertical exposure Cracking (RC and Otther) 2017 Underwater Inspect Pier #3R pile cap has a derive ELEMENT NAME Re Conc Pier Cap There are reinforced conc remaining chloride extract hairline cracks, hollow are pdf" for further details. Delaminatton/Spall/Pattcher See photos 21, 116-119 further details.	3 on: The exposed R (Gano Str re of 3.5' high 3 stion: crack 6' high ENV 3 rete caps at tion materia ba and spall d Ar3 and the atta 3	1,151.00 osed pile caps s d up to full-heigh reet Ramp). Piers gh. There is no c 1.00 n x 3/16" wide ex QUANTITY 388.00 t Piers #14 throughout (F is. See photos 2 310.00 ached file "07000 1.00	ft step out from it with varying s #3R, #5 an observed un ffi tending from UNITS ft uNITS ft agh #17. The Photos 21, 1 1, 116-119 ar ffi 1 Elem 234 ffi	CS 1 1,150.00 In the face of the p ing measurements d #9 exhibit expose dermining at any of 0.00 In the top of the pile QTY CS 1 50.00 Caps were recent 16). The caps and ind the attached file 0.00 Defect Table.pdf" for 0.00	CS 2 1.00 ier stems at varying from 2' (full-height) sed concrete tremie of the piers. 1.00 cap. QTY CS 2 313.00 ly repaired and are pedestals exhibit is e "070001 Elem 234 293.00 or 1.00	CS 3 0.00 widths from at Pier #5 to seals up to a 0.00 0.00 QTY CS 3 25.00 covered with solated Defect Table	CS 4 0.00 0.00 0.00 QTY CS 4 0.00
220 1130 ELEM 234	Re Conc Pile Cap/Ftig 2017 Underwater Inspection 10" wide to 18" wide and a 9.0' (full-height) at Pier #31 maximum vertical exposure Cracking (RC and Otther) 2017 Underwater Inspect Pier #3R pile cap has a derive ELEMENT NAME Re Conc Pier Cap There are reinforced conceremaining chloride extracted hairline cracks, hollow are pdf" for further details. Delaminatton/Spall/Pattchere See photos 21, 116-119 Exposed Rebar See photos 21, 116-119	3 on: The exposed R (Gano Str re of 3.5' high 3 ction: crack 6' high ENV 3 rete caps at tion materia a and spall d An3 and the atta 3 and the atta	1,151.00 osed pile caps s d up to full-heigh reet Ramp). Piers gh. There is no c 1.00 n x 3/16" wide ex QUANTITY 388.00 t Piers #14 throughout (F is. See photos 2 310.00 ached file "07000 1.00	ft step out from it with varying s #3R, #5 an observed un ffi tending from UNITS ft uNITS ft agh #17. The Photos 21, 1 1, 116-119 ar ffi 1 Elem 234 ffi	CS 1 1,150.00 In the face of the p ing measurements d #9 exhibit expose dermining at any of 0.00 In the top of the pile QTY CS 1 50.00 Caps were recent 16). The caps and ind the attached file 0.00 Defect Table.pdf" for 0.00	CS 2 1.00 ier stems at varying from 2' (full-height) sed concrete tremie of the piers. 1.00 cap. QTY CS 2 313.00 ly repaired and are pedestals exhibit is e "070001 Elem 234 293.00 or 1.00	CS 3 0.00 widths from at Pier #5 to seals up to a 0.00 0.00 QTY CS 3 25.00 covered with solated Defect Table	CS 4 0.00 0.00 0.00 QTY CS 4 0.00

) .		RIDOT	-		,	Washington B	07000 [,] Sridge Nortl			
	COT		Inspectio	on Rep	ort	Inspected I	Зу	AECOI			
D	Driven to get you there	Bridge	Condition	Poor		Last Inspe	Inspector:	07/22/2020			
1130	Cracking (RC and Otther)	3	12.00	ffi	0.00	12 00	0.00	0.00			
	See photos 21, 116-119 further details.	and the atta	ached file "07000)1 Elem 234	Defect Table.pdf	for					
ELEM	ELEMENT NAME	ENV	QUANTITY	UNITS	QTY CS 1	QTY CS 2	QTY CS 3	QTY CS 4			
00	Stirip Seal Exp Jointi	3	93.00	ft	0.00	88.00	5.00	0.00			
	This element was not ins previous Routine Inspect the left lanes of I-195 wes #3R for the Gano Street o	ion Report of tbound (Pho ff-ramp hav	dated 07/24/19: 1 oto 56). The port e been paved ov	There is a str tion of the jo ver (Photos s	rip seal joint in S int in the right I 54, 64).	Span #5 at the east anes of I-195 West	side of Pier #4 in bound and at Pier				
2310	Leakage	3	5.00	ffi	0.00	5.00	0.00	0.00			
	Routine Inspection Rep There is evidence of lea otos 65-66).			e underside o	due to failing join	it seal (Ph					
2330	Seal Damage	3	10.00	ffi	0.00	10 00	0.00	0.00			
	Routine Inspection Rep	ort dated 07	/24/2019:								
	The deck joint seal is lo	ose/sagging	/fallen along the	underside (F	Photos 65-66).						
2350	Debris Impactton	3	5.00	ffi	0.00	5.00	0.00	0.00			
	Routine Inspection Rep	ort dated 07	/24/2019:								
	The joint has full length (Photo 56).	partial debri	s impaction that	still allows fr	ee movement of	the joint					
2370	Mettal Detterioratton or Da	amagð	5.00	ffi	0.00	0.00	5.00	0.00			
	Routine Inspection Rep	ort dated 07	/24/2019:								
	The steel extrusion on t has a 3' long missing se		•		-	le lane					
LEM	ELEMENT NAME	ENV	QUANTITY	UNITS	QTY CS 1	QTY CS 2	QTY CS 3	QTY CS 4			
01	Pourable Jointi Seal	3	1,151.00	ft	507.00	544.00	85.00	15.00			
	This element was not ins previous Routine Inspect Abutment #1 and Piers #1 gore median in Spans #10 the on-going bridge cons patches and depressed p	ion Report of 1 through #7 6 and #17. A struction (Ph	dated 07/24/19: 1 /, on the east sic .II joints have be lotos 7, 42-44). T	There are po de of Piers # een paved ov The wearing #	urable joint seal 7 through #13, a rer in the right la surface along d	Is on the west side t East Abutment # anes of I-195 West eck joint edges ex	of West 2, and along the bound as part of hibits scattered				
2310	Leakage	3	344.00	ffi	0.00	344.00	0.00	0.00			
	Routine Inspection Rep										
	The joints exh bit scatte	ered evidence	e of leakage alor	ng the unders	sides (Photos 94	⊦, 104).					
2320	Seal Adhesion	3	300.00	ffi	0.00	200.00	85.00	15.00			
	Routine Inspection Report dated 07/24/2019: The pourable joint seals exhibit typical loss of seal adhesion up to full length with isolated										

	21,	ļ	RIDOT Inspectic	•		١	Washington E	0700 Bridge Nor
	COT	•	lispecie		UIT	Inspected E	-	AEC
L	Driven to get you there	Bridge	Condition	Poor		Last Inspec	Inspector: ction Date	07/22/20
LEM	ELEMENT NAME	ENV	QUANTITY	UNITS	QTY CS 1	QTY CS 2	QTY CS 3	QTY CS 4
10	Elastiomeric Bearing	3	401.00	each	136.00	190.00	75.00	0.00
	This element was not ins previous Routine Inspect and locations: - P/S conc Post-tensioned concrete I-girders in Spans #14 the #8 through #13 and at pie	tion Report d rete drop-in cantilever gi rough #18 - C	lated 07/24/19: T girder dapped e irder ends at the Concrete fascia	There are ela ends at the c e east wall o arches at th	astomeric bearin corbels in Spans f Pier #6 and the	ng pads for the follo #1 through #6 and west wall of Pier #	owing elements I #8 through #14 - #7 - P/S concrete	
2220	Alignmentt	3	4.00	each	0.00	0.00	4.00	0.00
	Routine Inspection Rep	ort dated 07/	/24/2019:					
	All measurements were	e recorded at	a temperature o	of 80-90 degi	rees Fahrenheit.			
	The drop-in girder bear typically in contraction are typically neutral or o	up to 1/2" (Pho	oto 147). The bea					
	The I-Girder bearings ir (Photo 206).	ו Spans #15	through #18 are	typically ne	utral or expanded	d up to ½"		
	The fascia arch bearing (Photo 209).	ıs in Spans #	1R through #3R	typically ne	utral or expande	d up to ½"		
230	Bulging, Splittng or Tearing	3	200.00	each	0.00	150.00	50.00	0.00
	Routine Inspection Rep	ort dated 07/	/24/2019:					
	The bearing pads exhit to moderate bulging an at top of bottom of pad	d isolated be	arings exhibit he		-			
240	Loss offi Bearing Area	3	61.00	each	0.00	40 00	21.00	0.00
	Routine Inspection Rep	ort dated 07/	/24/2019:					
	There are scattered loc and spalls above the be 136, 142, 147, 148, 163 Defect Table.pdf", "070 Table.pdf" for further de	earings reduc 3, 170, 205, 2 001 Elem 110	cing the bearing a 229-230 and the	area. See pl attached file	notos 107, 109, 1 es "070001 Elem	111, 115, 109		
	In Span #14 at Pier #14	I, Bearing 'F'	overhangs the p	oedestal ¾"	deep x 14" long ((Photo 205		
).					AT <i>i</i>	_	
LEM		ENV	QUANTITY	UNITS	QTY CS 1	QTY CS 2	QTY CS 3	QTY CS 4

			RIDOT	Bridge	•	1	Nashington B	07000 [.] ridgo Nortl
		In	spectio	on Repo	ort		-	•
	DOT		•	•		Inspected E	By Inspector:	AECO
	Driven to get you there	Bridge C	ondition	Poor		Last Inspec	· ·	07/22/202
515	Stteel Prottecttve Coattng	3	132.00	sq.ffi	0.00	0.00	44.00	88.00
	Routine Inspection Rep	oort dated 07/24	/2019:					
	The bearings have a st moderate rust. Bearing		-			to		
	ELEM ELEMENT NA				QTY	QTY	QTY	QTY
					CS 1	CS 2	CS 3	CS 4
	3420 Peel/Bub/Crack(S	tl Prc 3	132.00	sq.ft	0.00	0.00	44.00	88.00
	Routine Inspection	Report dated 07	7/24/2019:					
	See 515 - Steel Pro	tective Coating	notes.					
1000	Corrosion	3	9.00	each	0.00	7.00	2.00	0.00
	Routine Inspection Rep							
	The bearings and anch and 'K' have heavy lam				-			
	pack rust between the l		-					
2220	Alignmentt	3	1.00	each	0.00	0.00	1.00	0.00
	Routine Inspection Rep	oort dated 07/24	/2019:					
240	Loss offi Bearing Area	3	1.00	each	0.00	0.00	1.00	0.00
2240	Loss offi Bearing Area Routine Inspection Rep			each	0.00	0.00	1.00	0.00
2240		port dated 07/24	/2019: east corner 4"				1.00	0.00
	Routine Inspection Rep Bearing 'K' is undermin the west edge 16" wide	port dated 07/24	/2019: east corner 4"				1.00 QTY CS 3	0.00 QTY CS 4
2240 ELEM	Routine Inspection Rep Bearing 'K' is undermin the west edge 16" wide	port dated 07/24 and at the north a x up to 1" long	/2019: east corner 4" (Photo 248).	' wide x 4" lo	ng x 2" deep and QTY	d along QTY	QTY	QTY
ELEM	Routine Inspection Rep Bearing 'K' is undermin the west edge 16" wide ELEMENT NAME Fixed Bearing This element was not ins previous Routine Inspect limited access for full ins full width x ½" high x 6" /	eort dated 07/24 ed at the north e x up to 1" long ENV 3 spected per the tion Report dat spection due to deep gaps ben	A/2019: east corner 4" (Photo 248). QUANTITY 11.00 scope of this ed 07/24/19: T bearing restr eath the beari	' wide x 4" lo UNITS each special insp 'here are fixe aints in plac ng restraints	ng x 2" deep and QTY CS 1 0.00 Dection. The follo ed steel bearing: e at the west fac s at the west fac	d along QTY CS 2 8.00 owing has been re s in Span #7 at Pie ce of each bearing	QTY CS 3 3.00 tained from the r #7 that have . There are up to	QTY CS 4
:LEM 13	Routine Inspection Rep Bearing 'K' is undermin the west edge 16" wide ELEMENT NAME Fixed Bearing This element was not ins previous Routine Inspect limited access for full ins full width x ½" high x 6" have light to moderate ac	eort dated 07/24 ed at the north e x up to 1" long ENV 3 spected per the tion Report date spection due to deep gaps bene ccumulation of	A/2019: east corner 4" (Photo 248). QUANTITY 11.00 scope of this ed 07/24/19: T bearing restr eath the beari sand and deb	' wide x 4" lo UNITS each special insp here are fixe aints in plac ng restraints pris (Photo 2	ng x 2" deep and QTY CS 1 0.00 Dection. The follo ed steel bearing: e at the west fac at the west fac 11).	d along QTY CS 2 8.00 owing has been re s in Span #7 at Pie ce of each bearing ce (per rehab plans	QTY CS 3 3.00 tained from the r #7 that have . There are up to). The bearings	QTY CS 4 0.00
:LEM 13	Routine Inspection Rep Bearing 'K' is undermin the west edge 16" wide ELEMENT NAME Fixed Bearing This element was not ins previous Routine Inspect limited access for full ins full width x ½" high x 6" have light to moderate access Stteel Prottecttve Coatting	eort dated 07/24 ed at the north e x up to 1" long ENV 3 spected per the tion Report date spection due to deep gaps bene ccumulation of	A/2019: east corner 4" (Photo 248). QUANTITY 11.00 scope of this ed 07/24/19: T bearing restr eath the beari sand and deb 110.00	' wide x 4" lo UNITS each special insp 'here are fixe aints in plac ng restraints	ng x 2" deep and QTY CS 1 0.00 Dection. The follo ed steel bearing: e at the west fac s at the west fac	d along QTY CS 2 8.00 owing has been re s in Span #7 at Pie ce of each bearing	QTY CS 3 3.00 tained from the r #7 that have . There are up to	QTY CS 4
LEM 13	Routine Inspection Rep Bearing 'K' is undermin the west edge 16" wide ELEMENT NAME Fixed Bearing This element was not ins previous Routine Inspect limited access for full ins full width x ½" high x 6" have light to moderate ac	eort dated 07/24 ed at the north e x up to 1" long ENV 3 spected per the tion Report date spection due to deep gaps bene ccumulation of	A/2019: east corner 4" (Photo 248). QUANTITY 11.00 scope of this ed 07/24/19: T bearing restr eath the beari sand and deb 110.00	' wide x 4" lo UNITS each special insp here are fixe aints in plac ng restraints pris (Photo 2	ng x 2" deep and QTY CS 1 0.00 Dection. The follo ed steel bearing: e at the west fac at the west fac 11).	d along QTY CS 2 8.00 owing has been re s in Span #7 at Pie ce of each bearing ce (per rehab plans	QTY CS 3 3.00 tained from the r #7 that have . There are up to). The bearings	QTY CS 4 0.00
LEM	Routine Inspection Rep Bearing 'K' is undermin the west edge 16" wide ELEMENT NAME Fixed Bearing This element was not ins previous Routine Inspect limited access for full ins full width x ½" high x 6" have light to moderate access Stteel Prottecttve Coatting	eort dated 07/24 ed at the north e x up to 1" long ENV 3 spected per the tion Report date spection due to deep gaps bene ccumulation of 3 port dated 07/24 e a steel protec	A/2019: east corner 4" (Photo 248). QUANTITY 11.00 scope of this ed 07/24/19: T bearing restr eath the beari sand and deb 110.00 A/2019: tive coating with	' wide x 4" lo UNITS each special insp There are fixe aints in plac ng restraints oris (Photo 2' sq.ffi	ng x 2" deep and QTY CS 1 0.00 Dection. The follo ed steel bearing: e at the west fact at the west fact 11). 0.00	d along QTY CS 2 8.00 owing has been re s in Span #7 at Pie ce of each bearing ce (per rehab plans 0.00	QTY CS 3 3.00 tained from the r #7 that have . There are up to). The bearings	QTY CS 4 0.00
LEM 13	Routine Inspection Rep Bearing 'K' is undermin the west edge 16" wide ELEMENT NAME Fixed Bearing This element was not ins previous Routine Inspect limited access for full ins full width x ½" high x 6" have light to moderate act Stteel Prottecttve Coattng Routine Inspection Rep The fixed bearings have	eort dated 07/24 ed at the north ex up to 1" long ENV 3 spected per the tion Report date spection due to deep gaps bene ccumulation of 3 oort dated 07/24 e a steel protec 11). Bearings 'A	A/2019: east corner 4" (Photo 248). QUANTITY 11.00 scope of this ed 07/24/19: T bearing restr eath the beari sand and deb 110.00 A/2019: tive coating wi X, 'B', 'J' and 'I	' wide x 4" lo UNITS each special insp here are fixe aints in plac ng restraints oris (Photo 2' sq.ffi ith areas of p K' have no pa	ng x 2" deep and QTY CS 1 0.00 Dection. The follo ed steel bearing: e at the west fact at the west fact 11). 0.00	d along QTY CS 2 8.00 owing has been re s in Span #7 at Pie ce of each bearing ce (per rehab plans 0.00	QTY CS 3 3.00 tained from the r #7 that have . There are up to). The bearings	QTY CS 4 0.00
LEM 13	Routine Inspection Rep Bearing 'K' is undermin the west edge 16" wide ELEMENT NAME Fixed Bearing This element was not ins previous Routine Inspect limited access for full ins full width x ½" high x 6" i have light to moderate ac Stteel Prottecttve Coatting Routine Inspection Rep The fixed bearings have moderate rust (Photo 2	e a steel protect about dated 07/24 e a steel protect about dated 07/24 e a steel protect americana about contracts about dated 07/24 e a steel protect americana about contracts americana about contracts about dated 07/24 e a steel protect americana about contracts americana ab	A/2019: east corner 4" (Photo 248). QUANTITY 11.00 scope of this ed 07/24/19: T bearing restr eath the beari sand and deb 110.00 A/2019: tive coating wi X, 'B', 'J' and 'I	' wide x 4" lo UNITS each special insp here are fixe aints in plac ng restraints oris (Photo 2' sq.ffi ith areas of p K' have no pa	ng x 2" deep and QTY CS 1 0.00 eection. The follo ed steel bearing: e at the west fac s at the west fac 11). 0.00 eeeling paint with aint remaining. QTY	d along QTY CS 2 8.00 owing has been re s in Span #7 at Pie ce of each bearing ce (per rehab plans 0.00 n light to QTY	QTY CS 3 3.00 tained from the r #7 that have . There are up to). The bearings 66.00	QTY CS 4 0.00 44.00
LEM 13	Routine Inspection Rep Bearing 'K' is undermin the west edge 16" wide ELEMENT NAME Fixed Bearing This element was not ins previous Routine Inspect limited access for full ins full width x ½" high x 6" (have light to moderate ac Stteel Prottective Coating Routine Inspection Rep The fixed bearings have moderate rust (Photo 2 ELEM ELEMENT NA	e a steel protect about dated 07/24 e a steel protect about dated 07/24	A/2019: east corner 4" (Photo 248). QUANTITY 11.00 scope of this ed 07/24/19: T bearing restr eath the beari sand and deb 110.00 A/2019: tive coating wi X, 'B', 'J' and 'I QUANTITY 110.00	' wide x 4" lo UNITS each special insp here are fixe aints in plac ng restraints oris (Photo 2' sq.ffi ith areas of p K' have no pa UNITS	ng x 2" deep and QTY CS 1 0.00 Dection. The follo ed steel bearing: e at the west fac at the west fac 11). 0.00 Decling paint with aint remaining. QTY CS 1	d along QTY CS 2 8.00 owing has been re s in Span #7 at Pie ce of each bearing te (per rehab plans 0.00 n light to QTY CS 2	QTY CS 3 3.00 tained from the r #7 that have . There are up to). The bearings 66.00 66.00	QTY CS 4 0.00 44.00 QTY CS 4
LEM 13	Routine Inspection Rep Bearing 'K' is undermin the west edge 16" wide ELEMENT NAME Fixed Bearing This element was not ins previous Routine Inspect limited access for full ins full width x ½" high x 6" have light to moderate ac Stteel Prottecttve Coatting Routine Inspection Rep The fixed bearings hav moderate rust (Photo 2 ELEM ELEMENT NA 3420 Peel/Bub/Crack(S	eort dated 07/24 led at the north a x up to 1" long ENV 3 spected per the tion Report date spection due to deep gaps bene ccumulation of 3 bort dated 07/24 e a steel protect (11). Bearings 'A ME ENV t/ Prc 3 Report dated 07	A/2019: east corner 4" (Photo 248). QUANTITY 11.00 scope of this ed 07/24/19: T bearing restr eath the beari sand and deb 110.00 A/2019: tive coating wi X', 'B', 'J' and 'I QUANTITY 110.00 A/2019:	' wide x 4" lo UNITS each special insp here are fixe aints in plac ng restraints oris (Photo 2' sq.ffi ith areas of p K' have no pa UNITS	ng x 2" deep and QTY CS 1 0.00 Dection. The follo ed steel bearing: e at the west fac at the west fac 11). 0.00 Decling paint with aint remaining. QTY CS 1	d along QTY CS 2 8.00 owing has been re s in Span #7 at Pie ce of each bearing te (per rehab plans 0.00 n light to QTY CS 2	QTY CS 3 3.00 tained from the r #7 that have . There are up to). The bearings 66.00 66.00	QTY CS 4 0.00 44.00 QTY CS 4



070001 Washington Bridge North

Inspected By

Inspector: Last Inspection Date AECOM 07/22/2020

Routine Inspection Report dated 07/24/2019:

The bearings and anchor bolts typically have light to moderate rust (Photo 211). Bearings 'A', 'B', 'J' and 'K' have heavy laminated rust on the bearings and anchor bolts.

Bridge Condition Poor

ELEM	ELEMENT NAME	ENV	QUANTITY	UNITS	QTY CS 1	QTY CS 2	QTY CS 3	QTY CS 4
321	Re Conc Approach Slab	3	2,352.00	sq.ft	0.00	2,352.00	0.00	0.00

This element was not inspected per the scope of this special inspection. The following has been retained from the previous Routine Inspection Report dated 07/24/19: The reinforced concrete approach slabs are concealed from view by bituminous concrete wearing surfaces (Photos 1-5, 67-71).

510	Wearing Surffiaces	3	2,352.00	sq.ffi	1,352.00	500.00	500.00	0.00
	Routine Inspection Re	eport dated 07	/24/2019:					

The wearing surfaces have moderate wheel line rutting with sealed and unsealed cracks throughout (Photos 67-71).

ELEM	ELEMENT NAME	ENV	QUANTITY	UNITS	QTY CS 1	QTY CS 2	QTY CS 3	QTY CS 4
3220	Crack (Wearing Surfac 3		2,352.00	sq.ft	1,352.00	500.00	500.00	0.00
	Routine Inspection Report da	ated 07/	/24/2019:					

See 510 - Wearing Surface notes.

ELEM	ELEMENT NAME	ENV	QUANTITY	UNITS	QTY CS 1	QTY CS 2	QTY CS 3	QTY CS 4
331	Re Conc Bridge Railing	3	3,808.00	ft	3,396.00	411.00	1.00	0.00

This element was not inspected per the scope of this special inspection. The following has been retained from the previous Routine Inspection Report dated 07/24/19: There are reinforced concrete bridge railings on both sides of the bridge in Spans #1 through #18 (Photo 42). Numerous sections of the railings at the deck joints were recently demolished and re-constructed as part of the bridge rehabilitation (Photo 74). There are scattered utility box covers along the interior faces of the bridge railings, many with broken covers (Photos 46-48, 273, 275). The condition of the tops of the pylons is included in this element.

1080	Delaminatton/Spall/Pattched Ar3	10.00	ffi	0.00	10 00	0.00	0.00

Routine Inspection Report dated 07/24/2019:

The bridge railings exhibit isolated minor edge spalls along the top of the railing (Photo 76). In Span #14 the north railing at Pier #14 has a 12" long x 6" high x 1" deep spall.

The pylons exhibit typical scattered hollow areas and shallow spalls (Photos 77-80).

1120	Effiorescence/Rustt Sttaining	g 3	1.00	ffi	0.00	0.00	1.00	0.00
	Routine Inspection Repo	ort dated 07/2	24/2019:					
	See 1130 – Cracking no	tes.						
1130	Cracking (RC and Otther)	3	351.00	ffi	0.00	351.00	0.00	0.00
	Routine Inspection Repo	ort dated 07/2	24/2019:					
	The bridge railings exhib	oit typical sca	ttered full height	hairline verti	cal cracks (Pho	to 75).		
	The pylons exhibit typica	al scattered c	racks and rust st	ains (Photos	; 77-80).			

7000	Damage	3	50.00	ffi	0.00	50 00	0.00	0.00
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Inspected By AECOM Inspector: Last Inspection Date

07/22/2020

Bridge Condition Poor Routine Inspection Report dated 07/24/2019:

The bridge railings ex	hibit random minor scrapes.

ELEM	ELEMENT NAME	ENV	QUANTITY	UNITS	QTY CS 1	QTY CS 2	QTY CS 3	QTY CS 4
8060	Scupper	3	27.00	(EA)	0.00	3.00	20.00	4.00

This element was not inspected per the scope of this special inspection. The following has been retained from the previous Routine Inspection Report dated 07/24/19: The scupper drainage grates along both shoulders of I-195 Westbound and along the north shoulder of the Gano Street Off-Ramp are fully clogged with sand and debris; only isolated grates remain partially open with clean drain pipe openings (Photos 82, 268). In Span #17 the drainage grate along the north shoulder is fully clogged and missing the drainage grate (Photo 269). The drain pipe at the end of Pier #17 has a disconnected section (Photo 270). The drain pipes on the interior of the Gano Street off-ramp box girders have been replaced with new PVC piping (Photo 192).

1000	Corrosion	3	4.00	(EA)	0.00	0.00	0.00	4.00
	Routine Inspection F	Report dated 07/2	24/2019:					

The scupper drain pipes on the underside of deck exhibit typical light to heavy rust. The Pier #3 drain pipes on the south face of Column 'A' and on the north face of Column 'F' have rust holes and leak onto members below (Photo 95).

ELEM	ELEMENT NAME	ENV	QUANTITY	UNITS	QTY CS 1	QTY CS 2	QTY CS 3	QTY CS 4
8107	Stieel Opn Girder/Beam ENDS	S 1	110.00	ft	0.00	0.00	110.00	0.00

15	Stteel Pro	ttecttve Coattng	1	1,615.00	sq.ffi	0.00	0.00	615.00	1,000.00
	See Elen Table.pdf	· •	notos 37-41,	, 108 and the attached fi	ttached file "	ached file "070001 Elem 10	07 Defect		
I	ELEM	ELEMENT NAME	ENV	QUANTITY	UNITS	QTY	QTY	QTY	QTY
						CS 1	CS 2	CS 3	CS 4
					-				
_EM	See E	l/Bub/Crack(Stl P Element 107 notes ENT NAME		1,615.00 41, 108 and th QUANTITY	sq.ft ne attached fi UNITS	0.00 ile "070001 Eler QTY	0.00 m 107 Defect Table QTY	615.00 e.pdf". QTY	1,000.00
_EM	See E	Element 107 notes	s, photos 37-	41, 108 and th	e attached f	ile "070001 Elei	m 107 Defect Table	e.pdf".	1,000.00 QTY CS 4
	See E	Element 107 notes	s, photos 37-	41, 108 and th	e attached f	QTY	m 107 Defect Table	e.pdf". QTY	QTY
LEM 213	See E ELEMI R/C Retiurr This eleme previous R West Abutr	Element 107 notes ENT NAME Wall I was not inspection	ENV 3 cted per the Report dat Abutment #	41, 108 and th QUANTITY 175.00 scope of this ed 07/24/19: Ti 2 and at both	UNITS (LF) special insp	QTY CS 1 0.00 ection. The foll	m 107 Defect Table QTY CS 2	QTY CS 3 25.00 etained from the ne north ends of	QTY CS 4

The top of the northwest return wall at West Abutment #1 has multiple edge spalls along

the cope up to 2" deep (Photo 264).

1120	Effiorescence/Rustt Sttaining	3	110.00	(LF)	0.00	85 00	25.00	0.00
		-		()				



070001 Washington Bridge North

Inspector:

AECOM

07/22/2020

Inspected By

Last Inspection Date

Bridge Condition Poor

Routine Inspection Report dated 07/24/2019:

e return walls have sid rust (Photos 264-26 acking (RC and Otther) utine Inspection Rep e 1120 Efflorescence affitt utine Inspection Rep ere is anti-graffiti pair 5-267). ELEMENT NAME wall, All Types element was not insp ous Routine Inspect Abutment #1 backw	3 3 ort dated 07 /Rust Stainin 3 ort dated 07 at and light g ENV 3 pected per t ion Report of	21.00 /24/2019: ng notes. 100.00 /24/2019: raffiti on the Wes QUANTITY 230.00 he scope of this	(LF) (LF) St Abutment UNITS (LF)	0.00	21 00	0.00 0.00 QTY CS 3 46.00	0.00 0.00 QTY CS 4
e 1120 Efflorescence affitt utine Inspection Rep ere is anti-graffiti pair 3-267). ELEMENT NAME wall, All Types element was not insp ous Routine Inspect Abutment #1 backw	ort dated 07. /Rust Stainin 3 ort dated 07. It and light g ENV 3 pected per t ion Report of	/24/2019: ng notes. 100.00 /24/2019: raffiti on the Wes QUANTITY 230.00 he scope of this	(LF) st Abutment UNITS (LF)	100.00 #1R return walls QTY CS 1	0.00 (Photos QTY CS 2	0.00 QTY CS 3	0.00 QTY CS 4
e 1120 Efflorescence affitt utine Inspection Rep ere is anti-graffiti pair 5-267). ELEMENT NAME wall, All Types element was not insp cous Routine Inspect Abutment #1 backw	/Rust Stainin 3 ort dated 07 ht and light g ENV 3 pected per t ion Report o	ng notes. 100.00 /24/2019: raffiti on the Wes QUANTITY 230.00 he scope of this	St Abutment UNITS (LF)	#1R return walls QTY CS 1	(Photos QTY CS 2	QTY CS 3	QTY CS 4
affitt utine Inspection Rep ere is anti-graffiti pair 5-267). ELEMENT NAME wall, All Types element was not ins pus Routine Inspect Abutment #1 backw	3 ort dated 07 nt and light g ENV 3 pected per t ion Report d	100.00 /24/2019: raffiti on the Wes QUANTITY 230.00 he scope of this	St Abutment UNITS (LF)	#1R return walls QTY CS 1	(Photos QTY CS 2	QTY CS 3	QTY CS 4
utine Inspection Rep ere is anti-graffiti pair 3-267). ELEMENT NAME wall, All Types element was not insp ous Routine Inspect Abutment #1 backw	ert dated 07 at and light g ENV 3 pected per t ion Report of	/24/2019: raffiti on the Wes QUANTITY 230.00 he scope of this	St Abutment UNITS (LF)	#1R return walls QTY CS 1	(Photos QTY CS 2	QTY CS 3	QTY CS 4
ere is anti-graffiti pair 5-267). ELEMENT NAME wall, All Types element was not ins pus Routine Inspect Abutment #1 backw	ENV BNV 3 pected per t	raffiti on the Wes QUANTITY 230.00 he scope of this	UNITS (LF)	QTY CS 1	QTY CS 2	CS 3	CS 4
S-267). ELEMENT NAME wall, All Types element was not insj pus Routine Inspect Abutment #1 backw	ENV 3 pected per t ion Report of	QUANTITY 230.00 he scope of this	UNITS (LF)	QTY CS 1	QTY CS 2	CS 3	CS 4
wall, All Types element was not insp ous Routine Inspect Abutment #1 backw	3 pected per t ion Report o	230.00 he scope of this	(LF)	CS 1	CS 2	CS 3	CS 4
element was not ins ous Routine Inspect Abutment #1 backw	pected per t ion Report o	he scope of this		-	-		-
element was not ins ous Routine Inspect Abutment #1 backw	ion Report o	-					0.00
laminatton/Spall/Pattche		80.00	(LF)	0.00	70 00	10.00	0.00
•							0.00
		nent #2 backwall	s exh bit rar	ndom hollow and i	minor		
alls up to 2' long x 2' l	high x 2" dee	ep.			-		
iorescence/Rustt Sttainin	g 3	23.00	(LF)	0.00	10 00	13.00	0.00
					rline		
acking (RC and Otther)	3	23.00	(LF)	0.00	0.00	23.00	0.00
					rline		
ELEMENT NAME	ENV	QUANTITY	UNITS	QTY CS 1	QTY CS 2	QTY CS 3	QTY CS 4
altic Jointi Matierial	3	1,438.00	(LF)	987.00	451.00	0.00	0.00
	st Abutment #1R and Ils up to 2' long x 2' l orescence/Rustt Sttainin st Abutment #1R and cical cracks, effloresc cking (RC and Otther) st Abutment #1R and tical cracks, effloresc ELEMENT NAME	st Abutment #1R and East Abutm Ils up to 2' long x 2' high x 2" dee orescence/Rustt Sttaining 3 st Abutment #1R and East Abutm tical cracks, efflorescence and ru cking (RC and Otther) 3 st Abutment #1R and East Abutm tical cracks, efflorescence and ru ELEMENT NAME ENV altic Jointi Matierial 3	st Abutment #1R and East Abutment #2 backwall Ils up to 2' long x 2' high x 2" deep. orescence/Rustt Sttaining 3 23.00 st Abutment #1R and East Abutment #2 backwall tical cracks, efflorescence and rust staining (Phote cking (RC and Otther) 3 23.00 st Abutment #1R and East Abutment #2 backwall tical cracks, efflorescence and rust staining (Phote st Abutment #1R and East Abutment #2 backwall tical cracks, efflorescence and rust staining (Phote ELEMENT NAME ENV QUANTITY altic Jointi Matierial 3 1,438.00	st Abutment #1R and East Abutment #2 backwalls exh bit rar Ils up to 2' long x 2' high x 2" deep. orescence/Rustt Sttaining 3 23.00 (LF) st Abutment #1R and East Abutment #2 backwalls exh bit typ tical cracks, efflorescence and rust staining (Photos 217, 222 cking (RC and Otther) 3 23.00 (LF) st Abutment #1R and East Abutment #2 backwalls exh bit typ tical cracks, efflorescence and rust staining (Photos 217, 222 cking (RC and Otther) 3 23.00 (LF) st Abutment #1R and East Abutment #2 backwalls exh bit typ tical cracks, efflorescence and rust staining (Photos 217, 222 ELEMENT NAME ENV QUANTITY UNITS altic Jointi Matierial 3 1,438.00 (LF)	st Abutment #1R and East Abutment #2 backwalls exh bit random hollow and it lls up to 2' long x 2' high x 2" deep. orescence/Rustt Sttaining 3 23.00 (LF) 0.00 st Abutment #1R and East Abutment #2 backwalls exh bit typical scattered hai tical cracks, efflorescence and rust staining (Photos 217, 222). 0.00 cking (RC and Otther) 3 23.00 (LF) 0.00 st Abutment #1R and East Abutment #2 backwalls exh bit typical scattered hai tical cracks, efflorescence and rust staining (Photos 217, 222). 0.00 st Abutment #1R and East Abutment #2 backwalls exh bit typical scattered hai tical cracks, efflorescence and rust staining (Photos 217, 222). 0.00 st Abutment #1R and East Abutment #2 backwalls exh bit typical scattered hai tical cracks, efflorescence and rust staining (Photos 217, 222). 0.00 st Abutment #1R and East Abutment #2 backwalls exh bit typical scattered hai tical cracks, efflorescence and rust staining (Photos 217, 222). 0.00 st Abutment NAME ENV QUANTITY UNITS QTY CS 1	st Abutment #1R and East Abutment #2 backwalls exh bit random hollow and minor Ils up to 2' long x 2' high x 2" deep. orescence/Rustt Sttaining 3 23.00 (LF) 0.00 10 00 st Abutment #1R and East Abutment #2 backwalls exh bit typical scattered hairline itcal cracks, efflorescence and rust staining (Photos 217, 222). cking (RC and Otther) 3 23.00 (LF) 0.00 0.00 st Abutment #1R and East Abutment #2 backwalls exh bit typical scattered hairline itcal cracks, efflorescence and rust staining (Photos 217, 222). 0.00 0.00 cking (RC and Otther) 3 23.00 (LF) 0.00 0.00 st Abutment #1R and East Abutment #2 backwalls exh bit typical scattered hairline tical cracks, efflorescence and rust staining (Photos 217, 222). 0.00 0.00 st Abutment #1R and East Abutment #2 backwalls exh bit typical scattered hairline tical cracks, efflorescence and rust staining (Photos 217, 222). 0.00 st Abutment MAME ENV QUANTITY UNITS QTY QTY cS 1 CS 2 1,438.00 (LF) 987.00 451.00	st Abutment #1R and East Abutment #2 backwalls exh bit random hollow and minor Ils up to 2' long x 2' high x 2" deep. orescence/Rustt Sttaining 3 23.00 (LF) 0.00 10 00 13.00 st Abutment #1R and East Abutment #2 backwalls exh bit typical scattered hairline tical cracks, efflorescence and rust staining (Photos 217, 222). cking (RC and Otther) 3 23.00 (LF) 0.00 0.00 23.00 st Abutment #1R and East Abutment #2 backwalls exh bit typical scattered hairline tical cracks, efflorescence and rust staining (Photos 217, 222). cking (RC and Otther) 3 23.00 (LF) 0.00 0.00 23.00 st Abutment #1R and East Abutment #2 backwalls exh bit typical scattered hairline tical cracks, efflorescence and rust staining (Photos 217, 222). ELEMENT NAME ENV QUANTITY UNITS QTY CS 1 CS 2 CS 3 altic Jointi Matierial 3 1,438.00 (LF) 987.00 451.00 0.00



070001 Washington Bridge North

Inspected By

Inspector: Last Inspection Date AECOM 07/22/2020

The asphaltic plug joints exhibit partial separations at joint edges and isolated cracks along the joints (Photos 60-61).

Routine Inspection Report dated 07/24/2019:

Bridge Condition Poor

ELEM	ELEMENT NAME	ENV	QUANTITY	UNITS	QTY CS 1	QTY CS 2	QTY CS 3	QTY CS 4
8335	Guardrail, Vehicular	3	700.00	(LF)	550.00	150.00	0.00	0.00

This element was not inspected per the scope of this special inspection. The following has been retained from the previous Routine Inspection Report dated 07/24/19: There are W-beam steel guardrails at the north side of the approaches for I-195 Westbound (Photos 3, 5). The Gano Street off-ramp has new W-beam steel guardrails attached to the interior faces of the bridge parapet that continue to the west approach (6, 50-54, 91). There is a new impact attenuator at the gore between I-195 Westbound and the Gano Street off-ramp (Photo 54).

515	Stteel Prottecttve Coattng	3	3,150.00	sq.ffi	1,800.00	0.00	1,350.00	0.00
	Routine Inspection Repor	t dated 07/24/2	2019:					
	The guardrails are galvar os 3, 5).	nized. The I-195	approach gua	rdrails have a	areas of light rust (F	Phot		
1000	Corrosion	3	100.00	(LF)	0.00	100.00	0.00	0.00
	Routine Inspection Repor	t dated 07/24/2	2019:					
	The I-195 approach guar	drails have area	as of light rust (Photos 3, 5).				
1020	Connectton	3	10.00	(LF)	0.00	10 00	0.00	0.00

Routine Inspection Report dated 07/24/2019:

The Gano Street off-ramp guardrails have scattered loose connection bolts to the parapets (Photo 89). The northwest approach guardrail at West Abutment #1R has missing

connection bolts at the 4th and 5th posts from the endpost (Photo 88).

7000	Damage	3	40.00	(LF)	0.00	40 00	0.00	0.00
	Douting Increatio	n Depart dated 07/	24/2010:					

Routine Inspection Report dated 07/24/2019:

The I-195 approach guardrails have 20' long areas of impact damage with leaning posts at the northwest and northeast guardrails (Photos 3, 90).

ELEM	ELEMENT NAME	ENV	QUANTITY	UNITS	QTY	QTY	QTY	QTY
					CS 1	CS 2	CS 3	CS 4
8336	Conc Bridge Parapeti	3	700.00	(LF)	350.00	320.00	30.00	0.00

This element was not inspected per the scope of this special inspection. The following has been retained from the	
previous Routine Inspection Report dated 07/24/19: The Gano Street off-ramp has a reinforced concrete bridge	
parapet with a single metal rail attached to the top face (Photos 6, 54, 73).	

1080 De	laminatton/Spall/Pattched Ar3	100.00	(LF)	0.00	100.00	0.00	0.00
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			RIDOT	Bridge)	١	Nashington I	07000 Bridge Nort
			Inspectio	on Rep	ort	Inspected E	-	AECO
	$\Box O I$					inopeoted E	Inspector:	
	Driven to get you there	Bridge	Condition	Poor		Last Inspec	tion Date	07/22/202
	Routine Inspection Rep	oort dated 07	/24/2019:					
	The parapets exhibit ty along the top of parape " long x up to 16" high h exposed rebars (Photo	t (Photo 84). nollow area v	The north parag	oet at midspa	in of Span #1R h	as an 8'-0		
	During the rehab projec parapet was hollow. Th guardrail posts because they removed the existi	ere's a crack e they were o	/seam that runs	about 1"-2" i	n. They didn't rep	place the		
1090	Exposed Rebar	3	100.00	(LF)	0.00	70 00	30.00	0.00
	Routine Inspection Rep	oort dated 07	/24/2019:					
	See 1080 Delaminatior	/Spall/Patch	ed Area notes					
1130	Cracking (RC and Otther)	3	150.00	(LF)	0.00	150.00	0.00	0.00
	Routine Inspection Rep	ort dated 07	/24/2019:					
ELEM	ELEMENT NAME	ENV	QUANTITY		OTV	QTY	OTV	071/
				UNITS	QTY CS 1	CS 2	QTY CS 3	QTY CS 4
3366	Rip Rap This element was not ins	3	1,000.00	sq.ft	CS 1 940.00	CS 2 30.00	CS 3 30.00	
8366 4000	Rip Rap This element was not ins previous Routine Inspect to 222). Above the high w block revetment to the ba embankment and there a Settlementt Routine Inspection Rep The rip rap has random several small sinkholes	3 pected per t tion Report of vater mark th ase of the ab re several sr 3 port dated 07 n missing stor	1,000.00 he scope of this dated 07/24/19: T here is a level ar butment. The rip mall sinkholes u 60.00 /24/2019: nes along the ch	sq.ft special insp There is rip r ea covered b rap has rand p to 12" dee sq.ffi annel embar	CS 1 940.00 Dection. The folk ap along the We by bituminous co dom missing sto p in the paveme 0.00 hkment and there o of the slope (Ph QTY	CS 2 30.00 owing has been re st Abutment #1R e oncrete pavement ones along the cha nt at the top of the 30 00 e are noto 222).	CS 3 30.00 tained from the mbankment (Pho and a sloped nnel slope. 30.00 QTY	CS 4 0.00 0.00 0.00
4000	This element was not ins previous Routine Inspect to 222). Above the high w block revetment to the ba embankment and there a Settlementt Routine Inspection Rep The rip rap has random several small sinkholes	3 pected per t tion Report of vater mark th ase of the ab re several sr 3 port dated 07 missing stor up to 12" de	1,000.00 he scope of this dated 07/24/19: 1 here is a level ar outment. The rip mall sinkholes u 60.00 /24/2019: hes along the ch eep in the pavem	sq.ft special insp There is rip r ea covered to rap has rand p to 12" dee sq.ffi annel embar ent at the top	CS 1 940.00 Dection. The follo ap along the We by bituminous co dom missing sto p in the paveme 0.00	CS 2 30.00 owing has been re st Abutment #1R e oncrete pavement ones along the cha nt at the top of the 30 00 e are noto 222).	CS 3 30.00 tained from the ombankment (Pho and a sloped nnel o slope. 30.00	CS 4 0.00
4000 ELEM	This element was not ins previous Routine Inspect to 222). Above the high w block revetment to the ba embankment and there a Settlementt Routine Inspection Rep The rip rap has random several small sinkholes ELEMENT NAME Slope Blocks This element was not ins previous Routine Inspect . The slope block protect hoto 222). ELEMENT NAME	3 pected per t tion Report of vater mark th ase of the ab re several sr 3 port dated 07. a missing stor up to 12" de ENV 3 pected per t tion Report of ion has more ENV	1,000.00 he scope of this dated 07/24/19: T here is a level ar outment. The rip mall sinkholes u 60.00 /24/2019: hes along the ch- hep in the paver QUANTITY 700.00 he scope of this dated 07/24/19: T tar deterioration QUANTITY	sq.ft special insp There is rip r ea covered to rap has rand p to 12" dee sq.ffi annel embar ent at the top UNITS sq.ft special insp There is a slo between the UNITS	CS 1 940.00 Dection. The folk ap along the We by bituminous co dom missing sto p in the paveme 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0	CS 2 30.00 owing has been re st Abutment #1R e oncrete pavement ones along the cha nt at the top of the 30 00 e are noto 222). QTY CS 2 0.00 owing has been re ment in front of W ht to moderate veg	CS 3 30.00 tained from the mbankment (Pho and a sloped nnel slope. 30.00 QTY CS 3 105.00 tained from the est Abutment #1F jetation growth (F QTY CS 3	2
00 E M 7	This element was not ins previous Routine Inspect to 222). Above the high w block revetment to the ba embankment and there a Settlementt Routine Inspection Rep The rip rap has random several small sinkholes ELEMENT NAME Slope Blocks This element was not ins previous Routine Inspect . The slope block protect hoto 222).	3 pected per t tion Report of vater mark th ase of the ab re several sr 3 port dated 07 n missing stor up to 12" de ENV 3 pected per t tion Report of ion has more ENV 3 ms between	1,000.00 he scope of this dated 07/24/19: 1 here is a level ar outment. The rip mall sinkholes u 60.00 /24/2019: hes along the ch beep in the paver QUANTITY 700.00 he scope of this dated 07/24/19: 1 tar deterioration QUANTITY 70.00 the steel girders	sq.ft special insp fhere is rip r ea covered to rap has rand p to 12" dee sq.ffi annel embar ent at the top UNITS sq.ft special insp fhere is a slop between the UNITS (EA) s in Span #7	CS 1 940.00 Dection. The follo ap along the We by bituminous co dom missing sto p in the paveme 0.00 Note: The slope (Ph QTY CS 1 595.00 Dection. The follo ped block revet e pavers and light QTY CS 1 13.00 Iabeled end diag	CS 2 30.00 owing has been re st Abutment #1R e oncrete pavement ones along the cha nt at the top of the 30 00 e are noto 222). QTY CS 2 0.00 owing has been re ment in front of W ht to moderate veg QTY CS 2 36.00	CS 3 30.00 tained from the mbankment (Pho and a sloped nnel slope. 30.00 QTY CS 3 105.00 tained from the est Abutment #1F letation growth (F QTY CS 3 17.00	CS 4 0.00 0.00 0.00 QTY CS 4 0.00

070001

07/22/2020

QTY



RIDOT Bridge Inspection Report

Washington Bridge North Inspected By AECOM

Inspector:

ast Inspection Da	ate
t	
	QTY CS 3

					CS 1	CS 2	CS 3	CS 4
34	410 Chalk(Steel Prot	ect Co 3	900.00	sq.ft	0.00	900.00	0.00	0.00
	See 515 - Steel Pr	rotective Coating r	notes.					
ELI	EM ELEMENT N	AME ENV	QUANTITY	UNITS	QTY	QTY	QTY	QTY
					CS 1	CS 2	CS 3	CS 4
34	420 Peel/Bub/Crack(Stl Prc 3	522.00	sq.ft	0.00	225.00	207.00	90.00
	See 515 - Steel Pr	rotective Coating r	notes.					
00	Corrosion	3	55.00	(EA)	0.00	35 00	16.00	4.00
10	Corrosion The end diaphragms of down to 1/8" remainin bottom flanges (Photo bearing stiffeners and	exhibit typical moo og thickness to top o 108). There is so	derate to heavy flanges and d attered pack r	y rust and co own to ¼" re	rrosion through maining thickne	out with ess to	16.00	4.00
	The end diaphragms of down to 1/8" remainin bottom flanges (Photo	exhibit typical moo g thickness to top o 108). There is so diaphragm conne	derate to heavy flanges and d attered pack r action plates.	y rust and co own to ¼" re ust up to 3/8	rrosion through maining thickne " thick between	out with ess to	16.00	4.00

Photo 109). Bay 'H' Diaphragm #1 has a two (2) mis-drilled bolt holes.

ELEM	ELEMENT NAME	ENV	QUANTITY	UNITS	QTY CS 1	QTY CS 2	QTY CS 3	QTY CS 4
8371	Conc Diaphragms	3	221.00	each	22.00	68.00	126.00	5.00

tl	nidspan diaphragm for drop-in girder hrough #6 and #8 through #14 - End 6 Gano Street off-ramp box girder interi	diaphragms and	a midspan d	liaphragm for I-g	, girders in Spans #	14 through #18 -	
s	Span #5, the east end of drop-in Girde ransfers loads to Girders 'A' and 'C' (er 'B' bears on a	n oversized L	-shaped diaphr	agm/transverse s	upport beam that	
t	o Span #5. The diaphragms were in v ocations where the diaphragm concre	arying stages of	f rehabilitatio	on during the ins	pection. There ar	e several	
	Scattered formwork remains in place		•	•	•		
S	cattered formwork remains in place	anoughout the	shage (i note			assemblies at	
t	he deck joints typically have the rest	rainer rod remov	ved (Photo 10)4). The diaphra	gms exhibit typic	al scattered	
tl h	•	rainer rod remov efflorescence ar	ved (Photo 10 nd rust staini)4). The diaphra ng, hairline to ½	gms exhibit typic ²" wide vertical cr	al scattered acks, random	
ti h c	he deck joints typically have the rest airline map cracks with and without	rainer rod remov efflorescence ar alls with and wit	ved (Photo 10 nd rust staini thout expose)4). The diaphra ng, hairline to ½ d and debonded	gms exhibit typic ²" wide vertical cr	al scattered acks, random	
ti h c	he deck joints typically have the rest nairline map cracks with and without concrete patches, hollow area and sp	rainer rod remov efflorescence ar alls with and wit	ved (Photo 10 nd rust staini thout expose)4). The diaphra ng, hairline to ½ d and debonded	gms exhibit typic ²" wide vertical cr	al scattered acks, random	0.00

1090	Exposed Rebar	3	12.00	each	0.00	6.00	1.00	5.00
	See photos 99 - 107 and details.	the attached fil	e "070001 Ele	m 8371 Defe	ct Table.pdf" for	further		
1120	Effiorescence/Rustt Sttaining	3	11.00	each	0.00	6.00	5.00	0.00
	See photos 99 - 107 and details.	the attached file	e "070001 Ele	m 8371 Defe	ct Table.pdf" for	further		

	21,		RIDOT Inspectio	Bridge		١	Washington	070001 Bridge North
	COT		mspecifi	лиср		Inspected E		AECOM
D	Driven to get you there	Bridge	e Condition	Poor		Last Inspec	Inspector: tion Date	07/22/2020
1130	Cracking (RC and Otther)	3	111.00	each	0.00	56 00	55.00	0.00
	See photos 99 - 107 ar details.	nd the attac	hed file "070001	Elem 8371 D	efect Table.pdf"	for further		
8368	Graffitt	3	100.00	each	0.00	100.00	0.00	0.00
	There are scattered are	as of heavy	r graffiti on the dia	aphragms (Pł	noto 99).			
ELEM	ELEMENT NAME	ENV	QUANTITY	UNITS	QTY CS 1	QTY CS 2	QTY CS 3	QTY CS 4
3398	Curb/sidewalks - Con This element was not ins previous Routine Inspect sides of the Gano Street	ion Report	dated 07/24/19: 1	There are cor	ncrete safety wa	alks and granite cu	rbs along both	0.00
3 398 	This element was not ins previous Routine Inspect sides of the Gano Street deep with vegetation grou	pected per ion Report off-ramp. Ti wth (Photo	the scope of this dated 07/24/19: ⊺ he safetywalks e	s special insp There are cor	pection. The foll acrete safety wa	owing has been re alks and granite cu	tained from the rbs along both	0.00
	This element was not ins previous Routine Inspect sides of the Gano Street (oected per ion Report off-ramp. Ti wth (Photo	the scope of this dated 07/24/19: T he safetywalks e 82). 698.00	s special insp There are cor xhibit typical	pection. The foll ncrete safety wa I heavy accumu	owing has been re alks and granite cu lation of dirt and d	tained from the rbs along both ebris up to 12"	
	This element was not inspervious Routine Inspect sides of the Gano Street deep with vegetation grou Delaminatton/Spall/Pattche	pected per ion Report off-ramp. Ti wth (Photo ed An1 ort dated 07 t scattered 1 vpical rust s curb has a	the scope of this dated 07/24/19: 1 he safetywalks e 82). 698.00 7/24/2019: hairline cracks ar taining and minor 15" wide x 2-1/2"	find general sc: r chipping thr long x 2" dee	0.00 0.00 aling ½" to 1" de oughout. In Spa p chip. The app	owing has been re alks and granite cu lation of dirt and d 698.00 eep (Photo n #3R roach	tained from the rbs along both ebris up to 12"	
	This element was not inspervious Routine Inspect sides of the Gano Street of deep with vegetation grou Delaminatton/Spall/Pattche Routine Inspection Rep The safety wa ks exhibi 83). The curbs exh bit ty near Pier #3R the south curbs are shifted up to 3	pected per ion Report off-ramp. The wth (Photo ed An1 ort dated 07 t scattered 1 vpical rust s curb has a 3" laterally v	the scope of this dated 07/24/19: 1 he safetywalks e 82). 698.00 7/24/2019: hairline cracks ar taining and minor 15" wide x 2-1/2"	find general sc: r chipping thr long x 2" dee	0.00 0.00 aling ½" to 1" de oughout. In Spa p chip. The app	owing has been re alks and granite cu lation of dirt and d 698.00 eep (Photo n #3R roach	tained from the rbs along both ebris up to 12"	
1080	This element was not insp previous Routine Inspect sides of the Gano Street deep with vegetation grou Delaminatton/Spall/Pattche Routine Inspection Rep The safety wa ks exhibit 83). The curbs exh bit ty near Pier #3R the south curbs are shifted up to 3 72-73).	pected per ion Report off-ramp. Th wth (Photo ed Ar1 ort dated 07 t scattered h rpical rust s curb has a 3" laterally v g 1	the scope of this dated 07/24/19: T he safetywalks e 82). 698.00 7/24/2019: hairline cracks ar taining and minou 15" wide x 2-1/2" vith typical gaps t 1.00	s special insp There are cor xhibit typical ffi ad general sca r chipping thr long x 2" dee up to 1" betwo	0.00 0.00 aling ½" to 1" de oughout. In Spa p chip. The app een curb section	owing has been re alks and granite cu lation of dirt and d 698.00 eep (Photo n #3R roach is (Photos	tained from the rbs along both ebris up to 12" 0.00	0.00
1080	This element was not inspervious Routine Inspect sides of the Gano Street of deep with vegetation grou Delaminatton/Spall/Pattche Routine Inspection Rep The safety wa ks exhibi 83). The curbs exh bit ty near Pier #3R the south curbs are shifted up to 3 72-73).	pected per ion Report off-ramp. The wth (Photo ed An1 ort dated 07 dated 07 dated 07 g 1 g 1 ort dated 07	the scope of this dated 07/24/19: T he safetywalks e 82). 698.00 7/24/2019: hairline cracks ar taining and minou 5" wide x 2-1/2" vith typical gaps u 1.00 7/24/2019:	s special insp There are cor xhibit typical ffi ad general sca r chipping thr long x 2" dee up to 1" betwo	0.00 0.00 aling ½" to 1" de oughout. In Spa p chip. The app een curb section	owing has been re alks and granite cu lation of dirt and d 698.00 eep (Photo n #3R roach is (Photos	tained from the rbs along both ebris up to 12" 0.00	0.00
1080	This element was not inspervious Routine Inspect sides of the Gano Street of deep with vegetation grow Delaminatton/Spall/Pattche Routine Inspection Rep The safety wa ks exhibit 83). The curbs exh bit ty near Pier #3R the south curbs are shifted up to 3 72-73). Effiorescence/Rustt Sttainin Routine Inspection Rep	pected per ion Report off-ramp. The wth (Photo ed An1 ort dated 07 dated 07 dated 07 g 1 g 1 ort dated 07	the scope of this dated 07/24/19: T he safetywalks e 82). 698.00 7/24/2019: hairline cracks ar taining and minou 5" wide x 2-1/2" vith typical gaps u 1.00 7/24/2019:	s special insp There are cor xhibit typical ffi ad general sca r chipping thr long x 2" dee up to 1" betwo	0.00 0.00 aling ½" to 1" de oughout. In Spa p chip. The app een curb section	owing has been re alks and granite cu lation of dirt and d 698.00 eep (Photo n #3R roach is (Photos	tained from the rbs along both ebris up to 12" 0.00	0.00

See 1080 Delamination/Spall/Patched Area notes.



Bridge Condition Poor

070001 Washington Bridge North

Inspected By

Inspector: Last Inspection Date

07/22/2020

AECOM

Work Candidattes

ssigned tto Municipalitty		ed tto Municipalitty		
Sttattus	Prioritty	Actton	Proposed	Nottes
Under Review	High		07/22/2020	Gano off-ramp box girder interiors: There is ponding water up to 6" deep at Pier 2R where the drain holes in the bottom flange remain clogged. This issue was reported last year during the routine inspection via phone and email and was also documented in the official inspection report. The drains should be cleared and cleaned to allow for proper drainage.
Under Review	High		07/22/2020	Gano off-ramp box girders: There are multiple unsecured points of access allowing pigeons into the box girders. One access hatch at Abutment 1R in Cell 'C' remains partially open, the access hole in the south web at Pier 3R has a detached screen, and Cell '1B' has a 12" wide x 12" long hole in the bottom flange. This has resulted in numerous areas of nesting pigeons with moderate to heavy debris which will impede future inspections if not cleaned. At a minimum the access points should be secured immediately.



070001 Washington Bridge North

Inspected By

AECOM

Inspector: Last Inspection Date

07/22/2020

Bridge Condition Poor

EquipmentAerial LiftImage: Second State S	Poison Ivy □ Heavy Vegetation □ Hurricane Evac Route ? □ Cones Yes Traffic Setup Req Yes Police Req Yes Night Insp Req No Signs Yes	Prep Time Crew Slize Varies Under Insp Vehicle Time Traffic Control Time 4 Mile Post Crew Days 20 Time Report Time Bucket Truck Time
Rigging I Floats I Climbing I Rail Mount Bucket Truck I Light Tower I	Yacht Club dock on Pier Rd locked hatches at W. Abut.	DI construction yard. Launch boat from E. Prov. Access Gano St Ramp box girder interiors via #1R with ladder. Access catwalks inside Pier #6 f the north overhang. The elect. room in E. Abut. om David Cluley(RIDOT).
Avg Curb Reveal North/East Avg Curb Reveal South/West Posted Weight Limit Posting Sign ? Post Signs Legible Post Sign Rec Adv Min Vert Clear Sign Min Vert Clear Signs Leg Min Vert Clear Post Vales Min Vert Clear Sign Rec Old Rating and Postings RR Mile Post US DOT/AAR No.	2.50 2.50 01 01 -1 01 13'-9" 01	TelephoneISewerICableIOilIFire AlarmIOH Lines PresentIWaterIGasIElectricIFiber OpticI